



SOLUTIONS FOR THE BUILT WORLD

Jefferson Barracks Bridge: A Case for Rehabilitation



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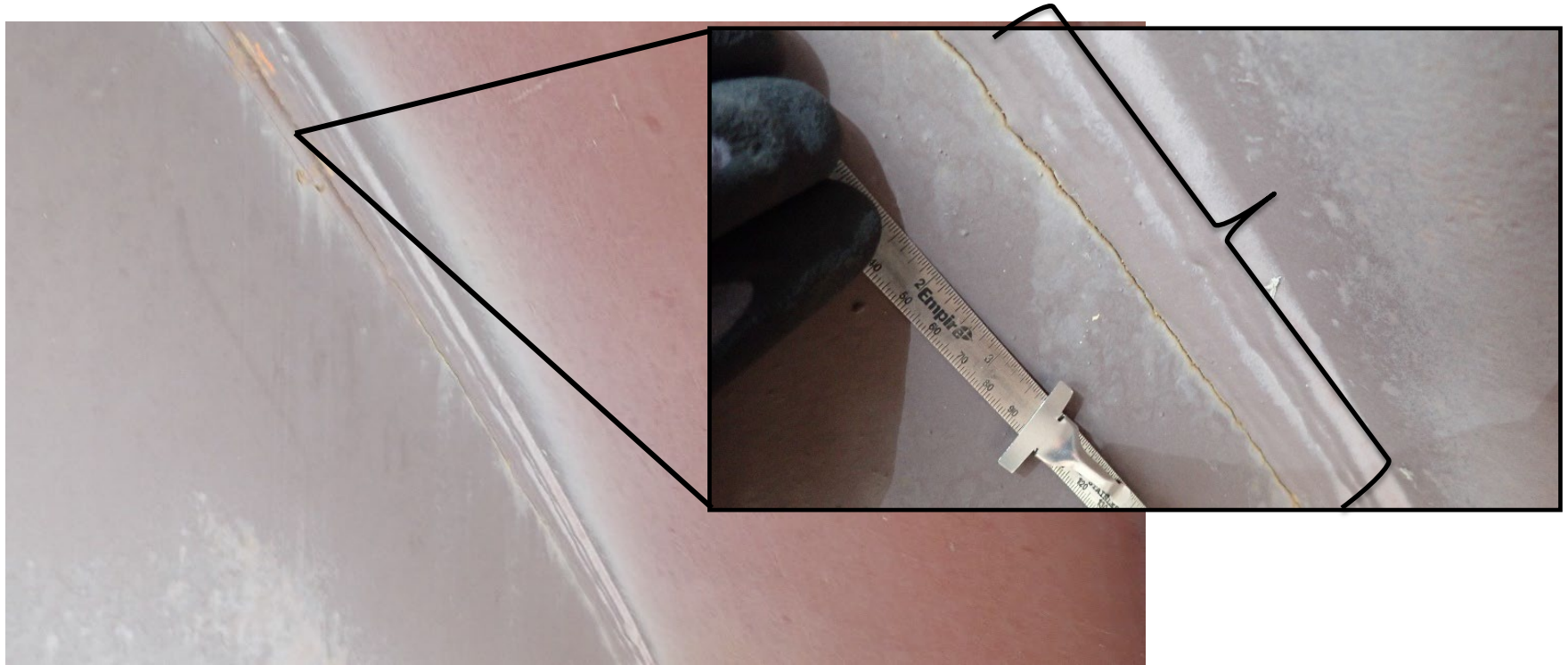
NSTM Inspection



Arch to Tie Girder Connection



Arch to Tie Girder Connection



Magnet Particle Testing



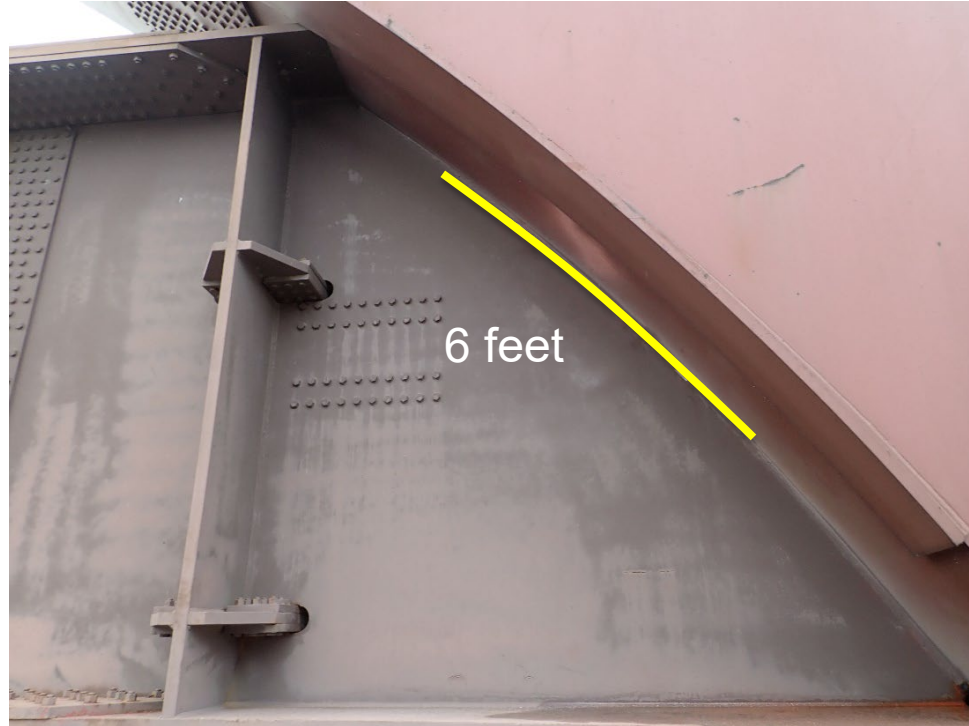
MT Inspection



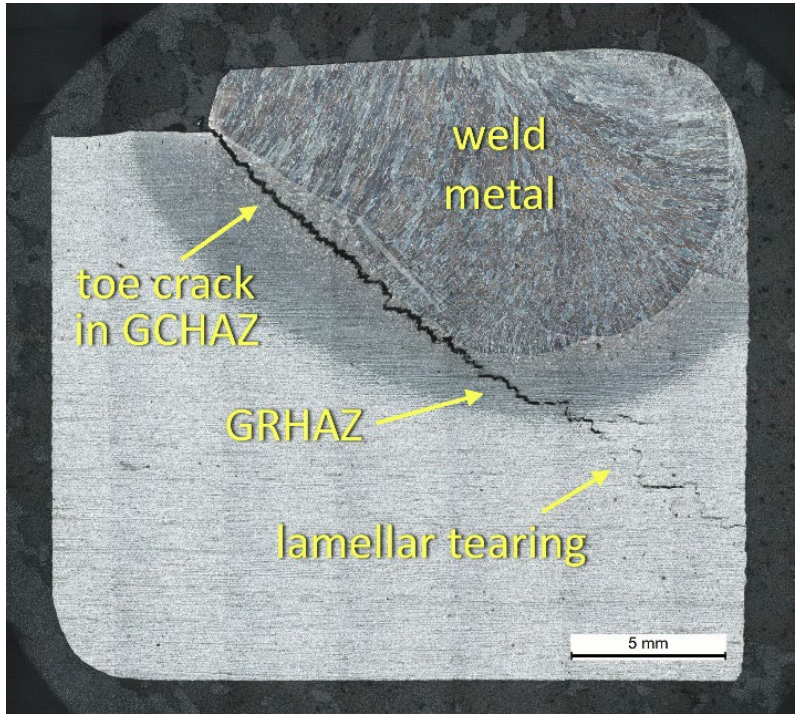
MT Inspection



MT Inspection



Crack Investigation



Weld Repairs



- 300 deg F preheat for 1 hour
- Shielded metal arc welding
- Low hydrogen consumables
- 3-hour bake out
- MT root pass and after cool down

Weld Repairs



Weld Repairs



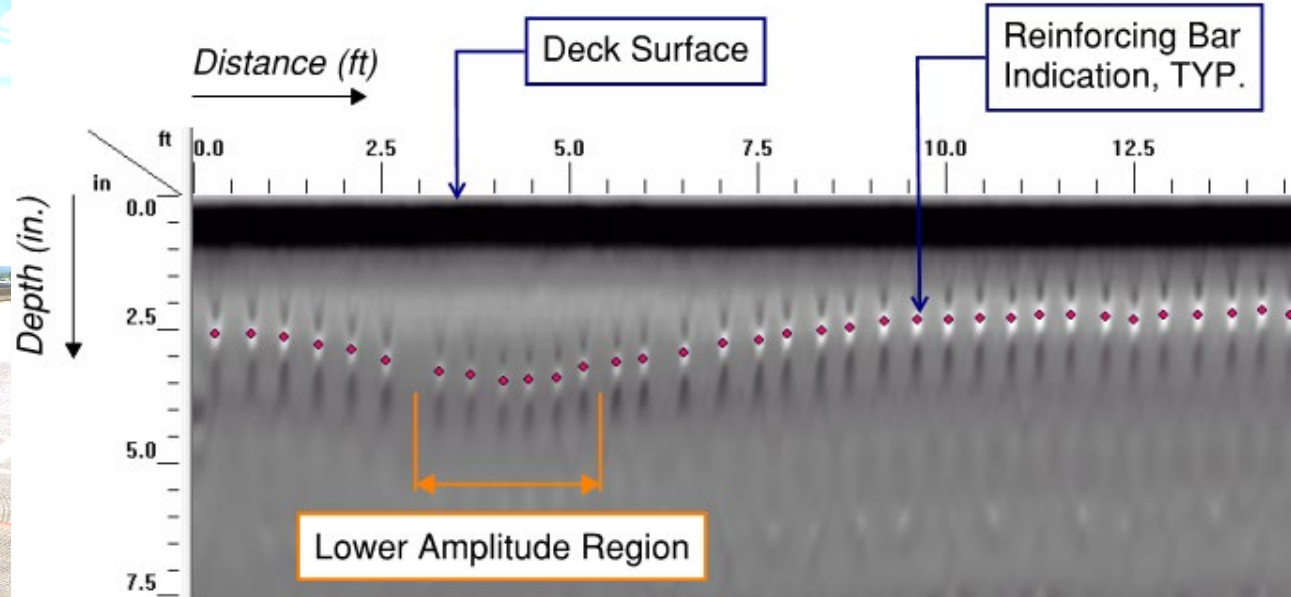
Investigation



Deck Sounding and Crack Mapping

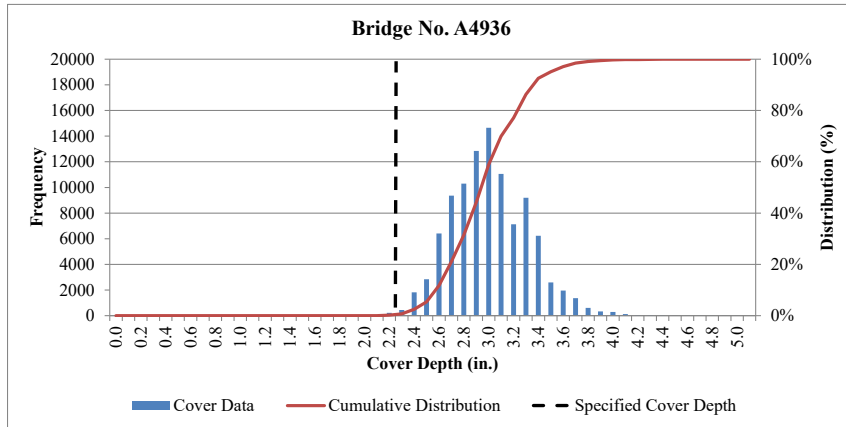


Reinforcing Steel Cover Measurements

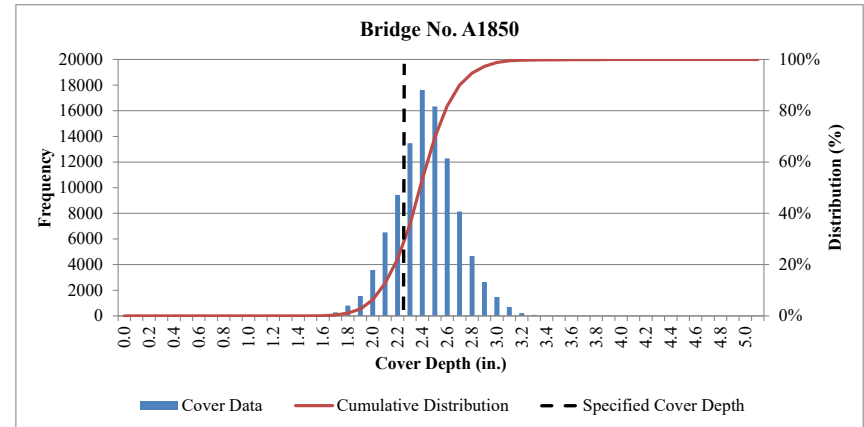


Reinforcing Bar Cover Measurements

Bridge A4936



Bridge A1850



Tensile Strength Pull-Off Testing



Concrete Cores

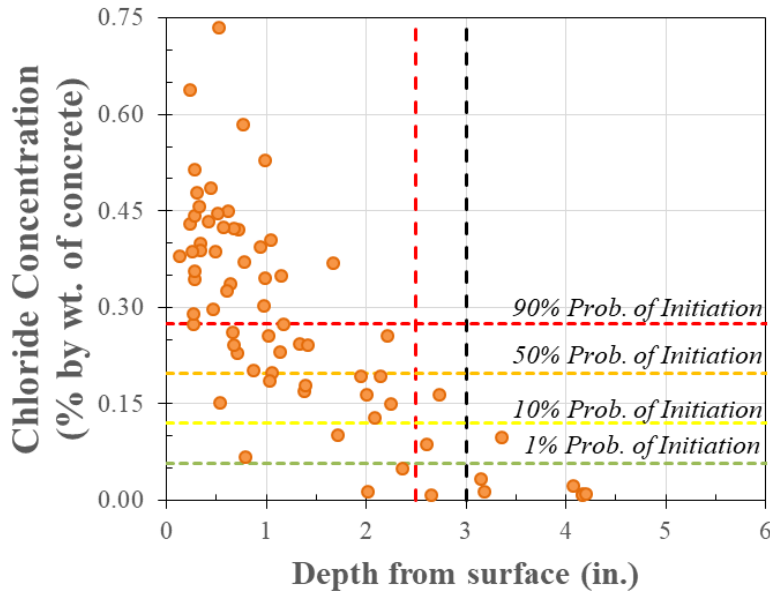


Condition of Epoxy

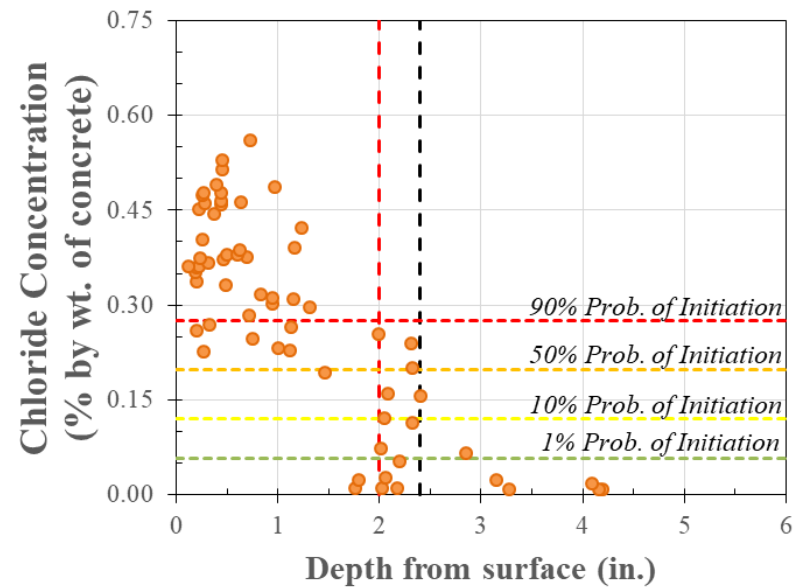


Chloride Concentrations

Bridge A4936



Bridge A1850



Bridge Deck Repair Options

- Deferred Rehabilitation
- Crack Repairs and Deck Surface Sealing
- Localized Concrete Patching and Sealing
- Hydro-demolition and Overlay installation
- Methyl methacrylate (MMA) or Epoxy Polymer Overlay
- PPC Overlay with Localized Concrete Patching

Deck Expansion Joints - Modular

Bridge A4936



Bridge A1850



Deck Expansion Joints - Finger

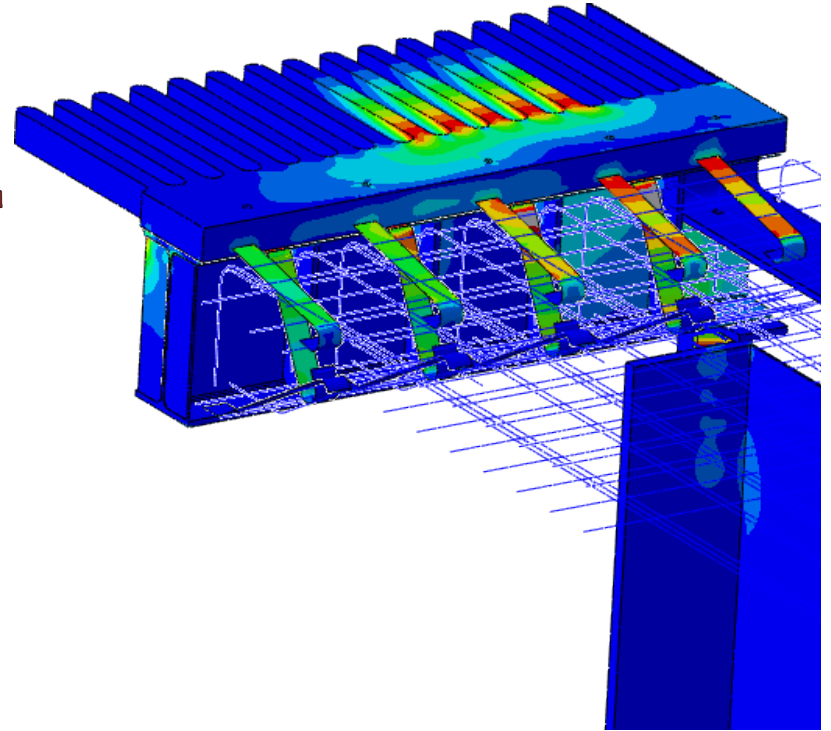
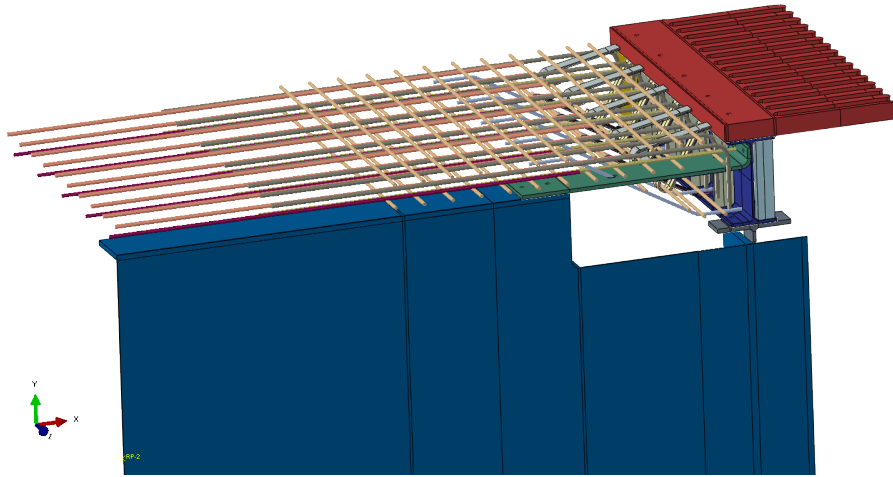
Bridge A4936



Bridge A1850

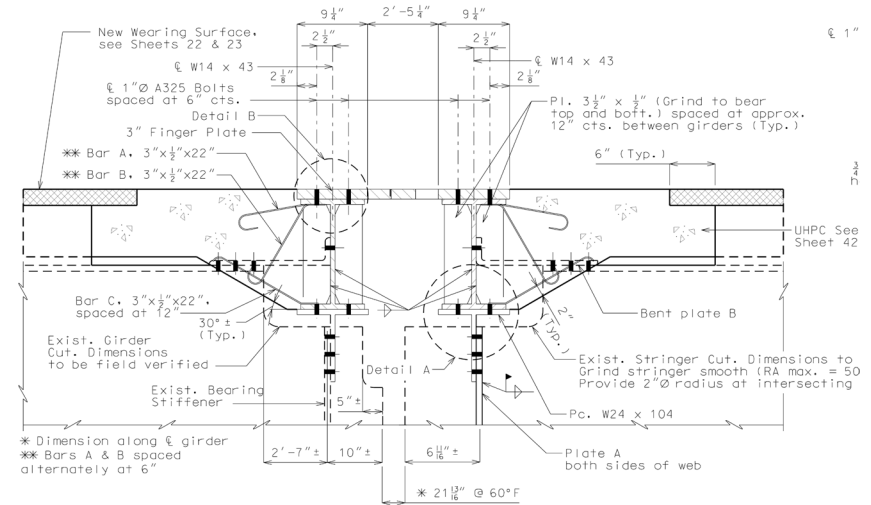


Deck Expansion Joints - Finger



Deck Expansion Joint Repairs

- Finger joints instead of modular joints
- Design heavy duty finger joint to prevent fatigue
- Use UHPC for joint headers
- Replace joint seals



Hanger Cable Corrosion



Hanger Cable Corrosion



Hanger Cable Repairs

- Replace cables
- Coat cables
- Metalize cables
 - Different abrasive media to achieve required bond

Assessment of Weathering Steel

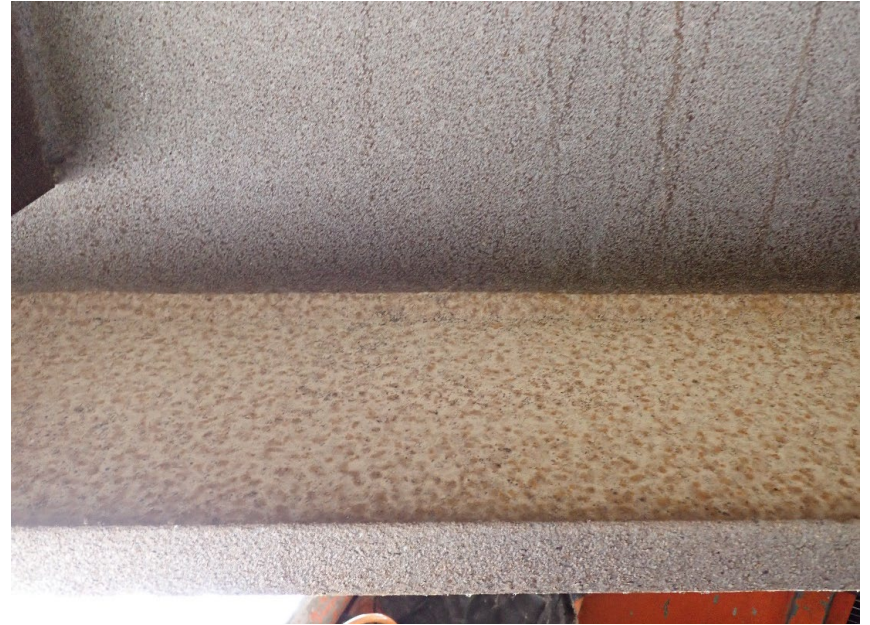


Condition of Weathering Steel

Failed Patina



Well-developed Patina



Condition of Steel Coating

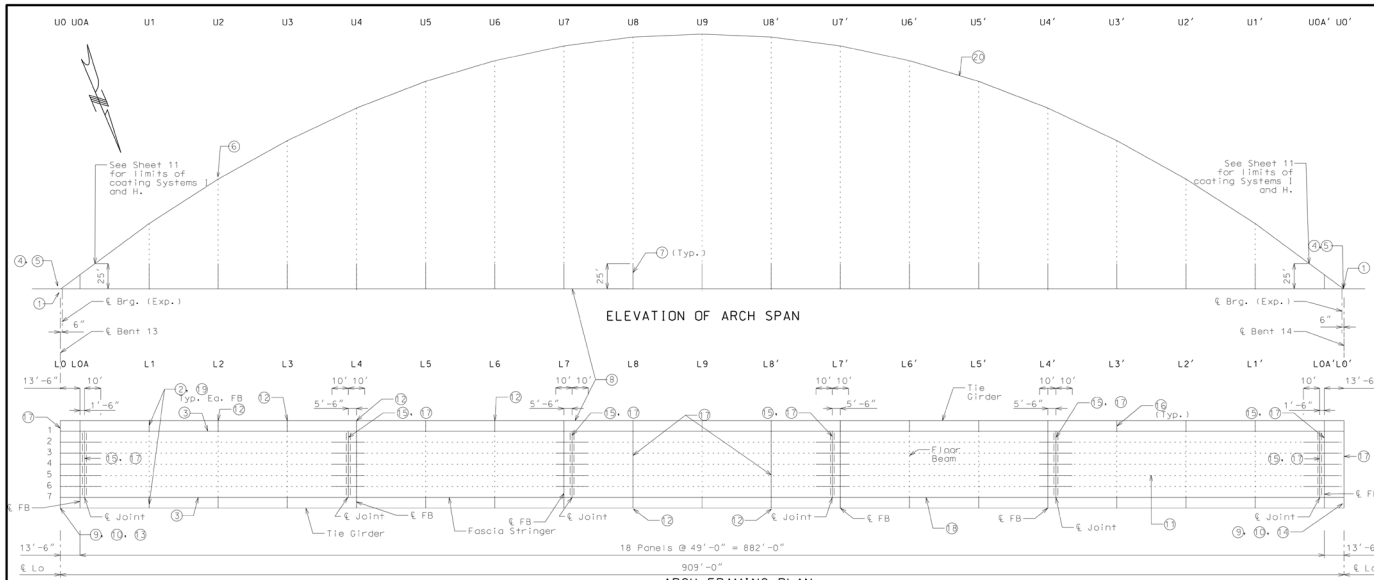


Condition of Steel Coating



Steel Coating Repairs

- Confirmed superstructure elements are weathering steel
- Apply System H to fascia girders including top side of bottom flange
- Apply System H to arch elements from 25 ft above to below deck
- Apply System I to arches and bracing 25 ft and above deck
- Coat lower interior portions of arches



ELEVATION OF ARCH SPAN

ARCH FRAMING PLAN

Retrofit/Repair	General Location	Description
①	Arch to tie girder connection	Weld inspection and repair at arch to tie girder connections. See Sheets 13-14 and 16.
②	Floor beam to tie girder connection	Weld inspection and repair at floor beam to tie girder connections. See Sheets 15-16.
③	Drainage truss and walkway	Repairs to misc. components. See Sheets 17 and 18.
④	Arch chamber interior	Stair removal. See Sheet 17.
⑤	Arch chamber door	Gasket replacement. See Sheet 16.
⑥	Partial access door	Pin replacement. See Sheet 19.
⑦	Hanger cables	Metallizing of lower 25 ft. See Sheet 11.
⑧	Tie girder	Clean & coat. See Sheet 11.
⑨	Reinforced concrete bent	Concrete repairs. See Sheet 8.
⑩	Reinforced concrete bent	Clearance gauge installation. See Sheet 9.

Retrofit/Repair	General Location	Description
⑪	Concrete deck and barrier	Concrete repairs, wearing surface and penetrating sealer installation. See Sheets 22 and 23.
⑫	Hanger cables	Replace hanger cables and remove and replace casting connections as part of weld inspection. See Sheets 15, 20-21.
⑬	Expansion joint, Bent No. 13	Replace existing modular joint with new finger joint. See Sheets 37-40, 42 and 45.
⑭	Expansion joint, Bent No. 14	Replace existing finger joint with new strip seal joint. See Sheets 37-41, 43 and 45.
⑮	Link joints	Replace existing joint seals with new preformed joint seals. See Sheet 46.
⑯	Floor beam ends	Clean & coat. See Sheet 11.
⑰	Floor beams and expansion devices	Clean & coat full length at joints and where indicated. See Sheet 11.
⑱	Stringer, exterior fascia	Clean & coat. See Sheet 11.
⑲	Walkway ladder repairs	Repair or replace ladder landing plates, each FB except L9. See Sheet 17.
⑳	Arch and struts	Clean & coat - System 1 from 25' above tie girder. System H below 25'. See Sheet 11.

LOCATION OF RETROFITS, REPAIRS & COATING LIMITS - TIED ARCH SPAN

Designed S.G.
Detailed L.S.
Checked J.C.M.

Note: This drawing is not to scale. Follow dimensions. Sheet No. 7 of 49

Notes:
Ⓢ Denotes location of retrofit or repair in Arch Span.

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

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Certificate of Authority
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Expires 10/31/2022

DATE PREPARED	10/1/2021
PROJECT SHEET NO.	255 MO BR 7
COUNTY	ST. LOUIS
JOB NO.	J613413
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO.	A18503

DESCRIPTION: SEE SHEET 11 FOR LIMITS OF COATING SYSTEMS I AND H.

Rehabilitation Project

- 2-Year project with A4936 closed first to avoid modular joint failure during two-way traffic.
- One bridge closed during work with two-way traffic on the other
- A1850 going second provided lead time for 24 new cables at hanger brackets with crack repairs
- KCI Construction Company winning bidder at \$49,997,800
- PPC for A4936 per plans and LMC for A1850 per alternate

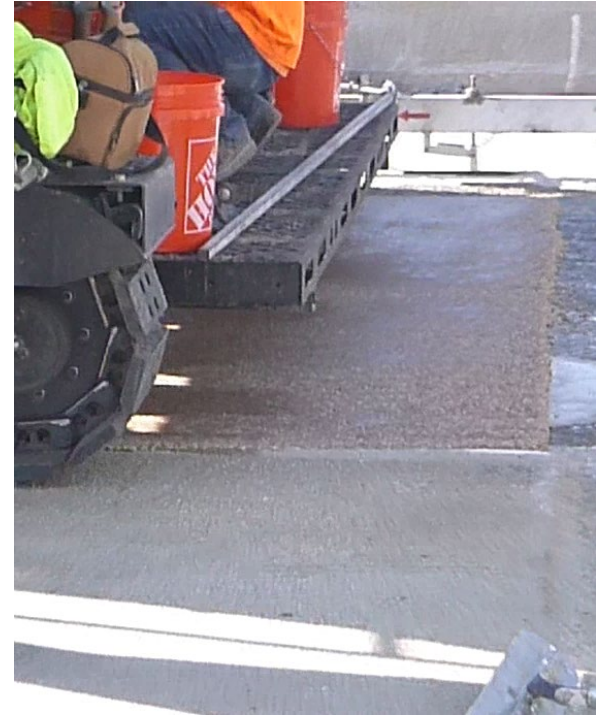
Arch Coating – Stage 1



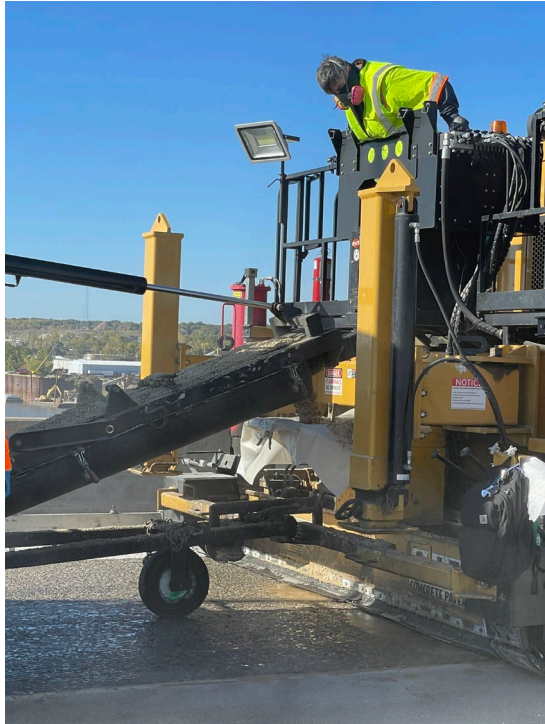
Arch Coating – Stage 2



PPC



PPC



PPC



PPC

- Partial deck patches used PPC and placed before overlay
- Hand placed on shoulders
- Drivable within 6 hrs.
- Speed of placement led contractor to switch to PPC for A1850 structure despite additional cost



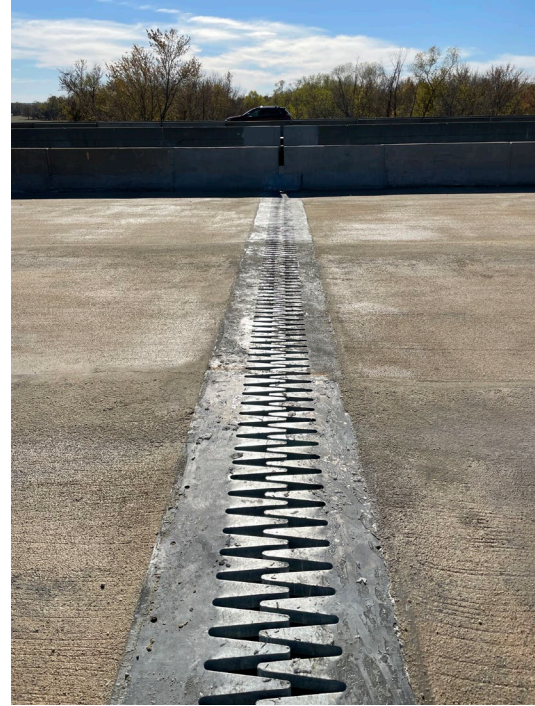
Joint Demolition



Heavy Duty Finger Joint



Bridge Deck Expansion Joints



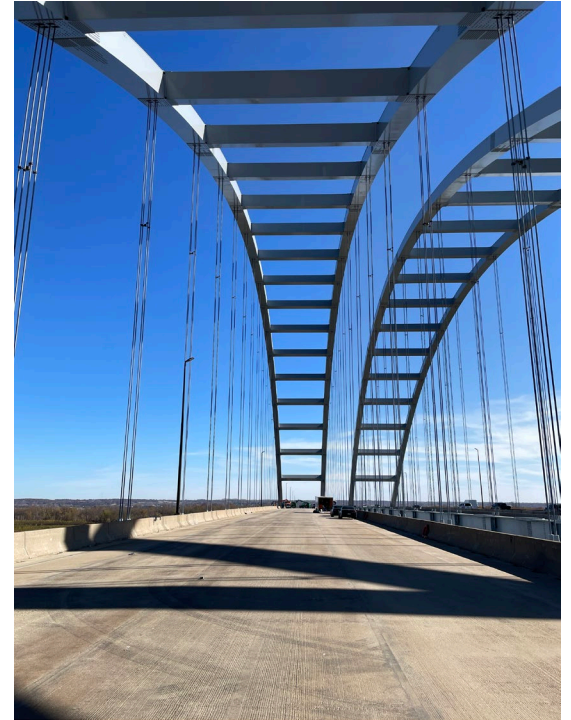
Hanger Cable Metalizing



Hanger Cable Metalizing



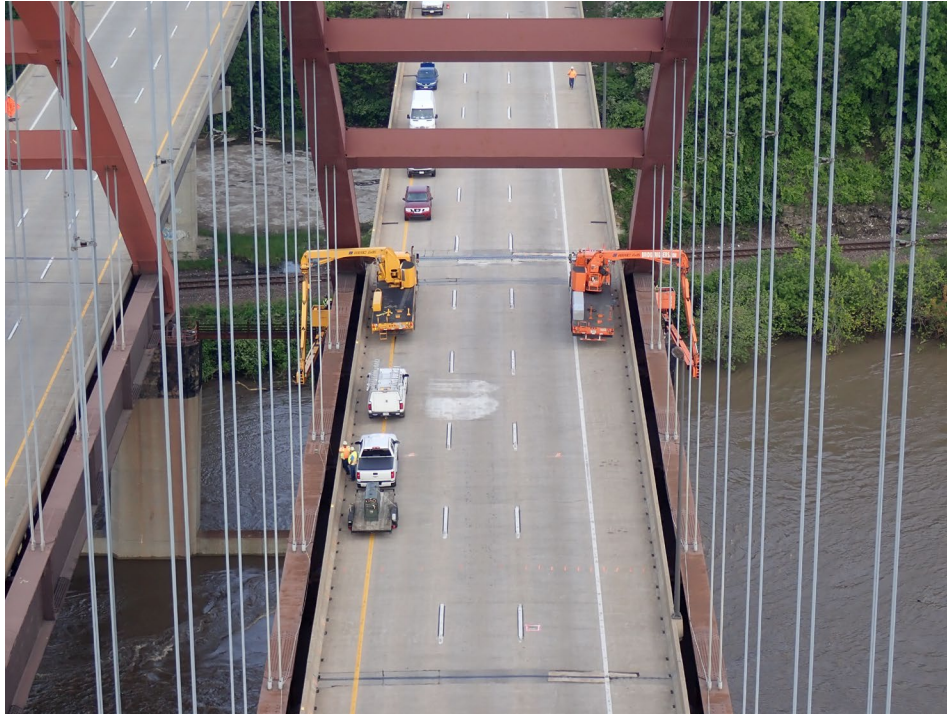
A Case for Rehabilitation

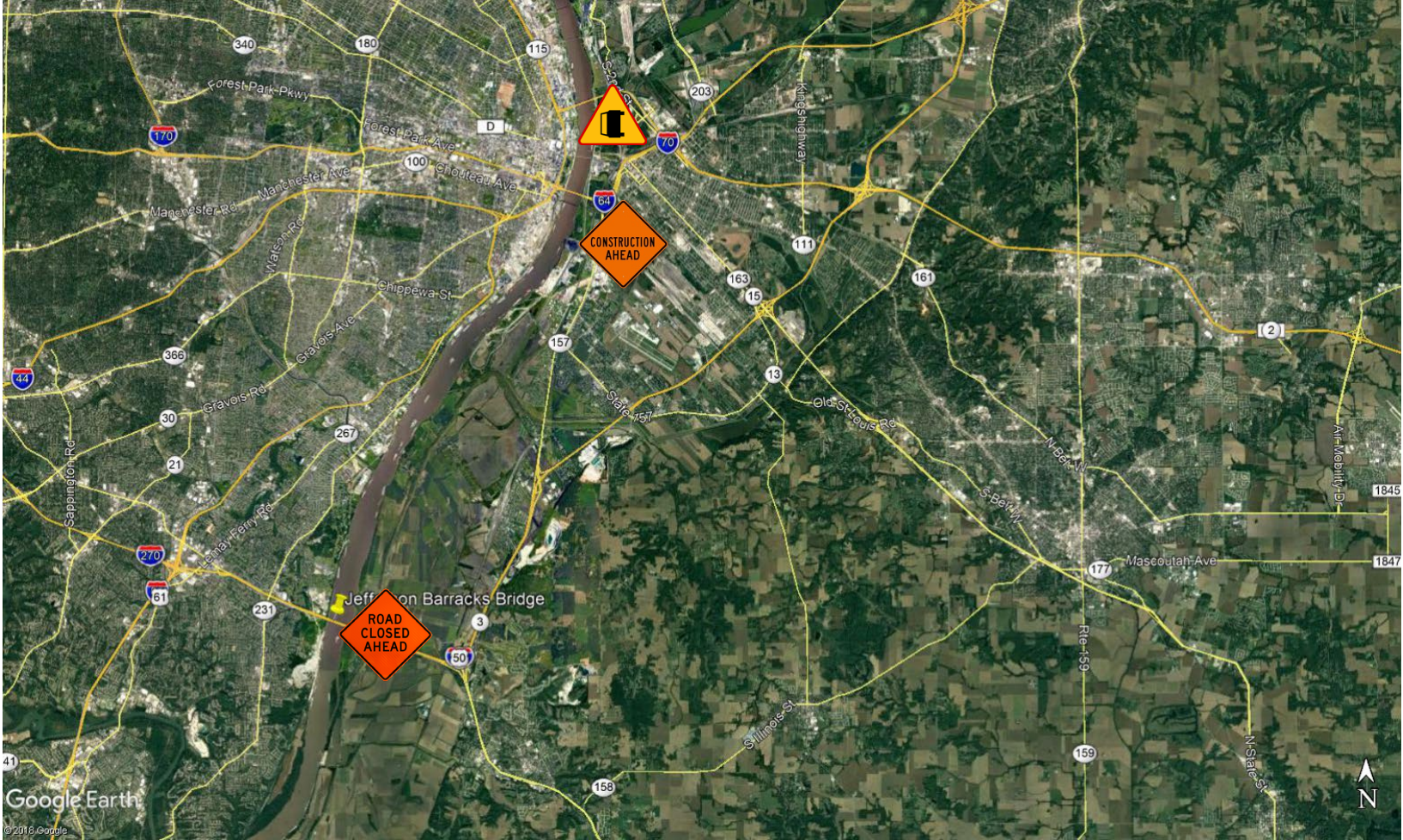


Questions?

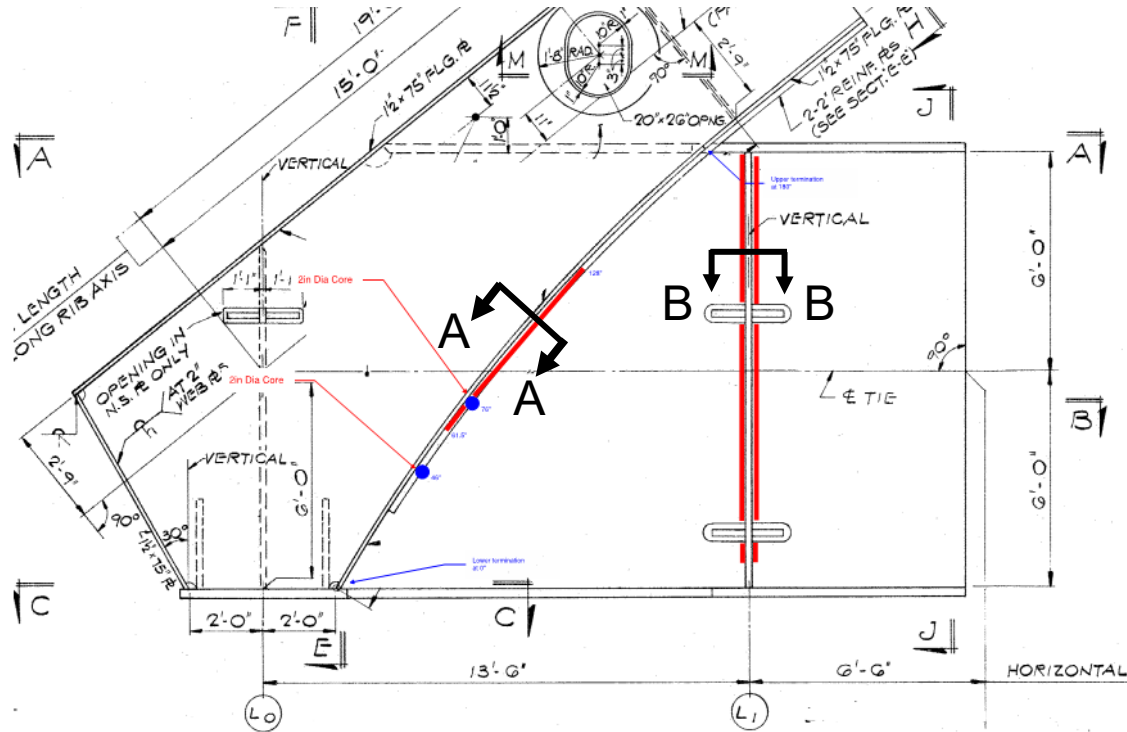


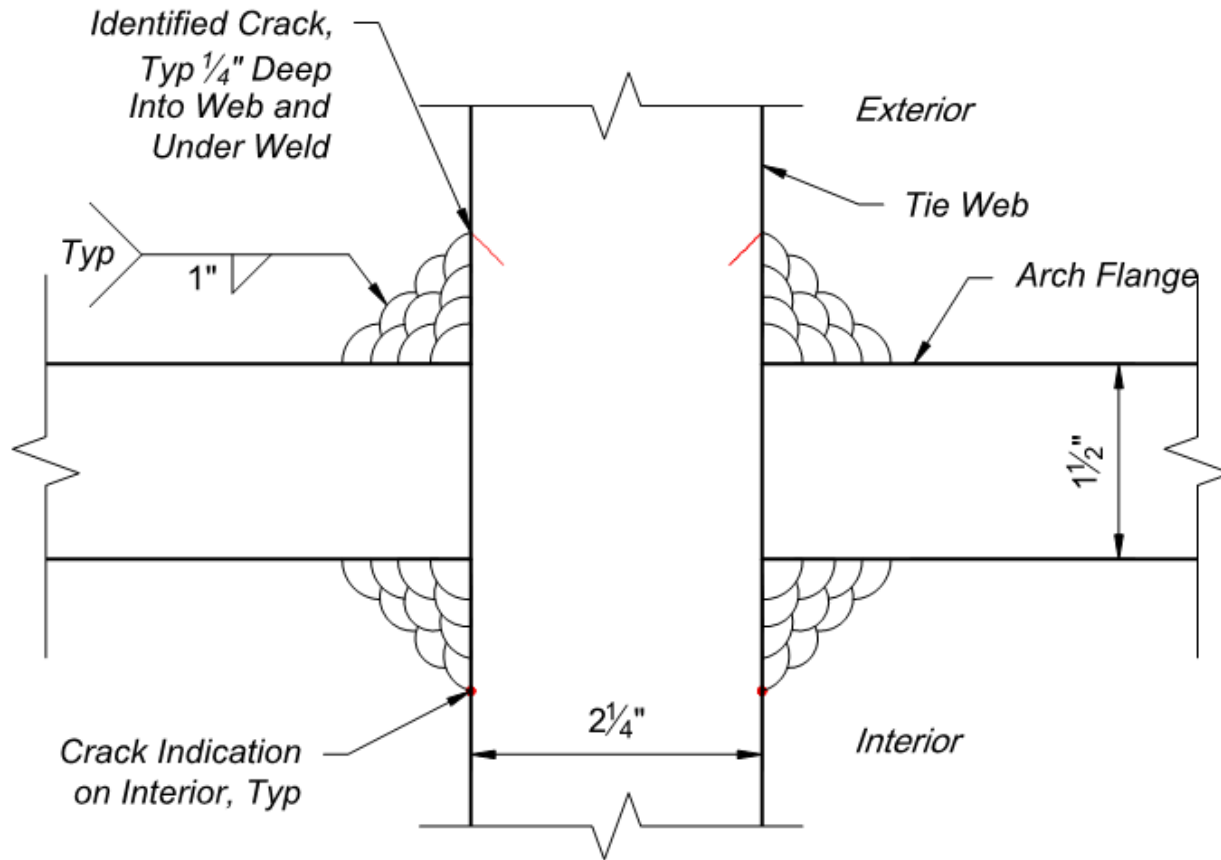
Bridge Closure



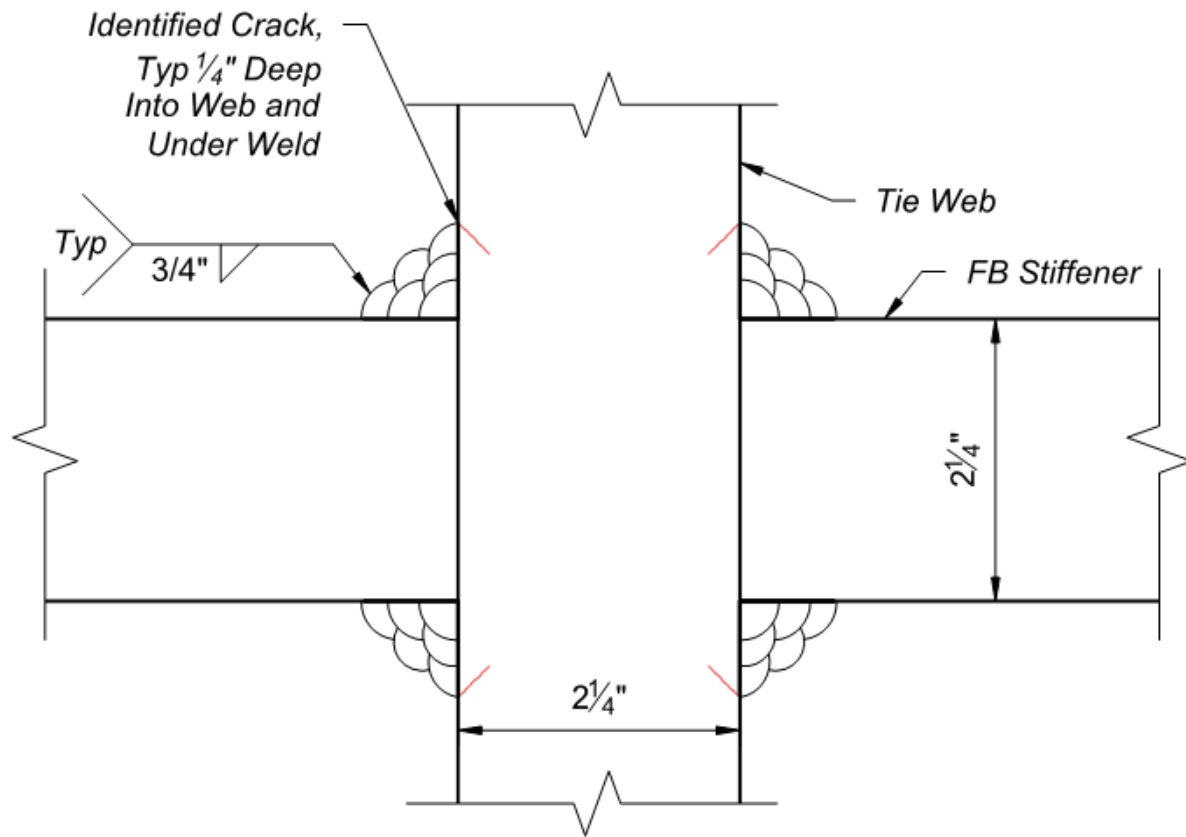


Tie Girder Connection Details





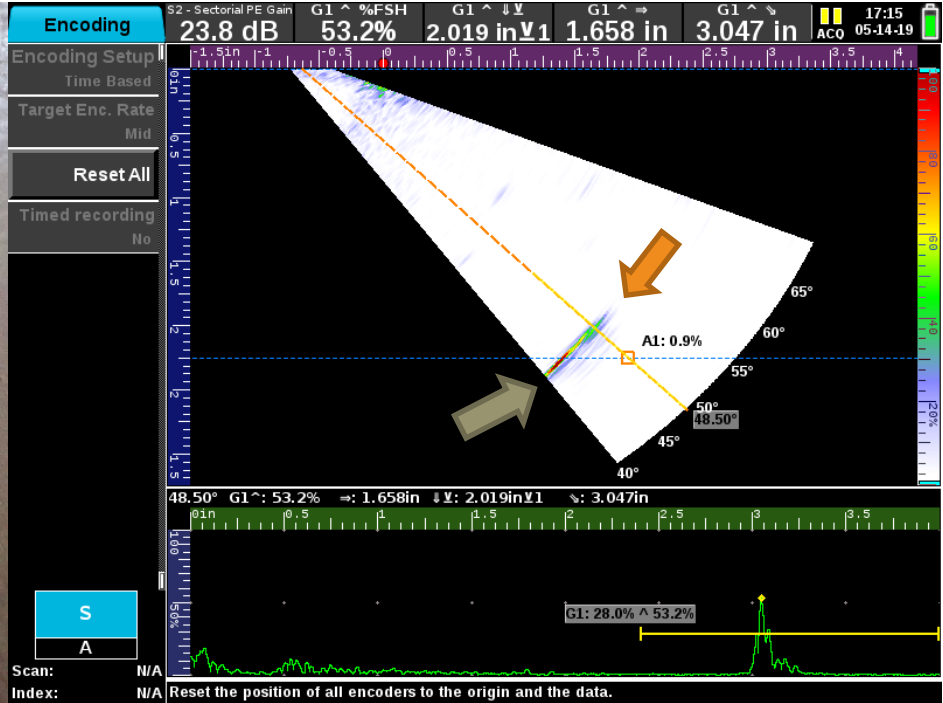
Section A-A



Section B-B

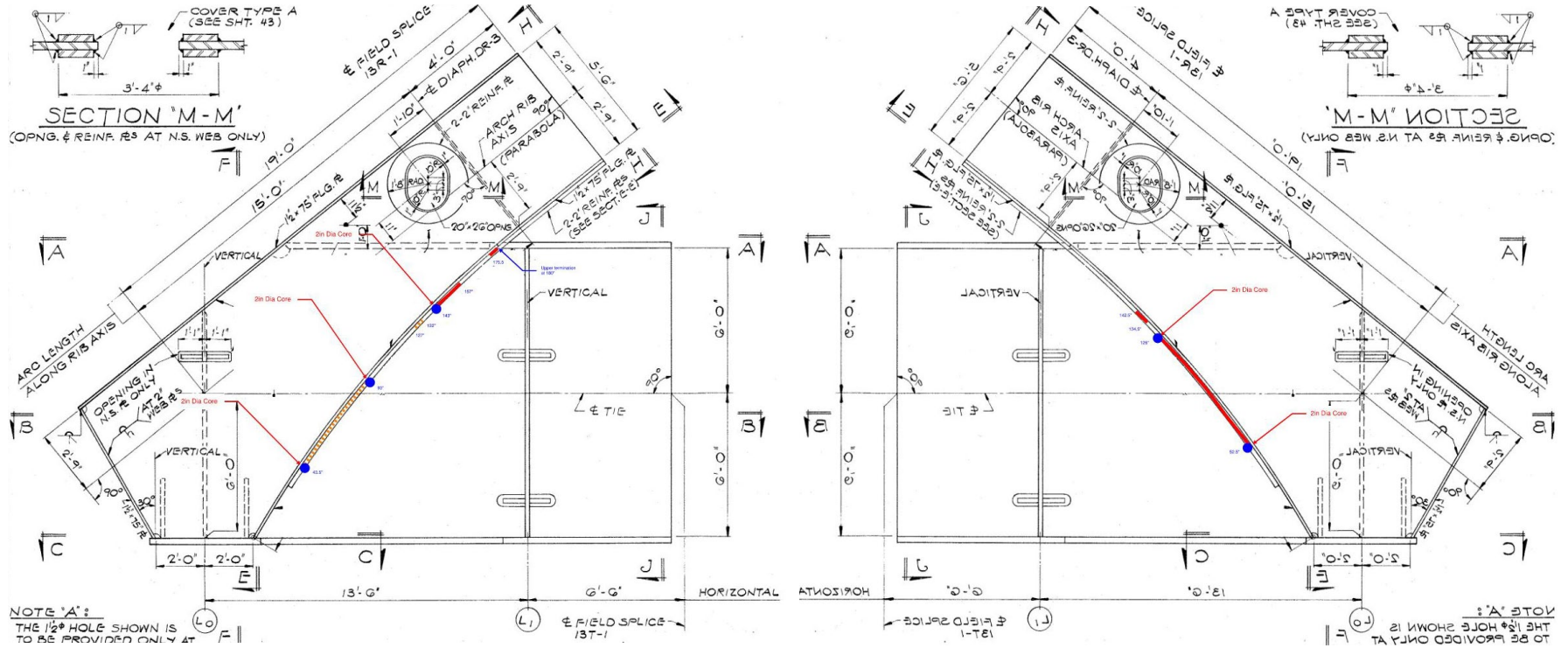
Nondestructive Evaluation of Cracks



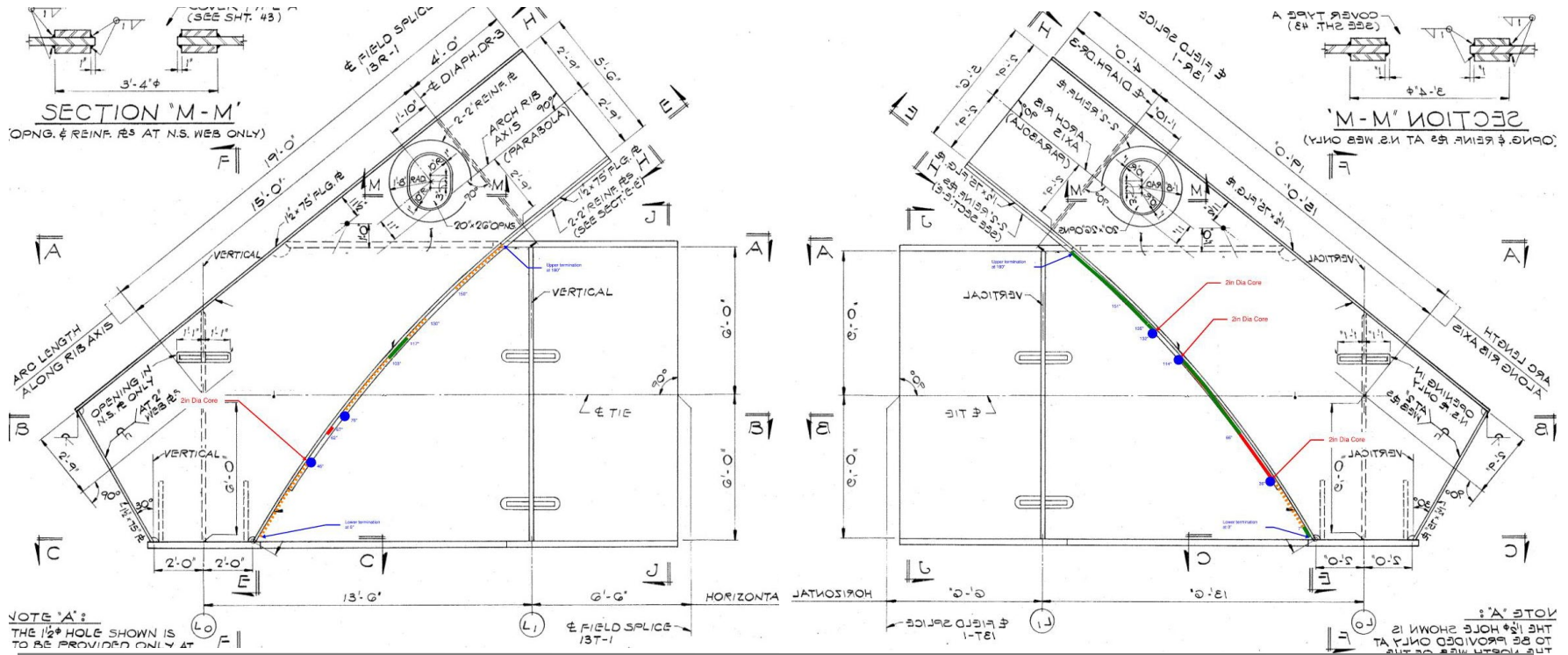




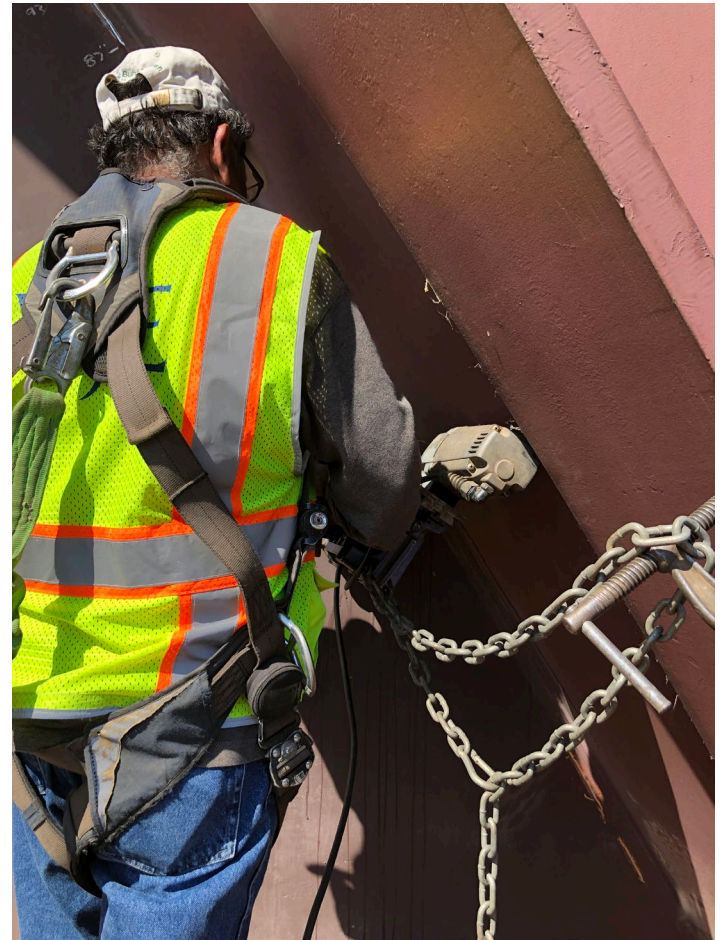
South Tie Girder (View looking North)



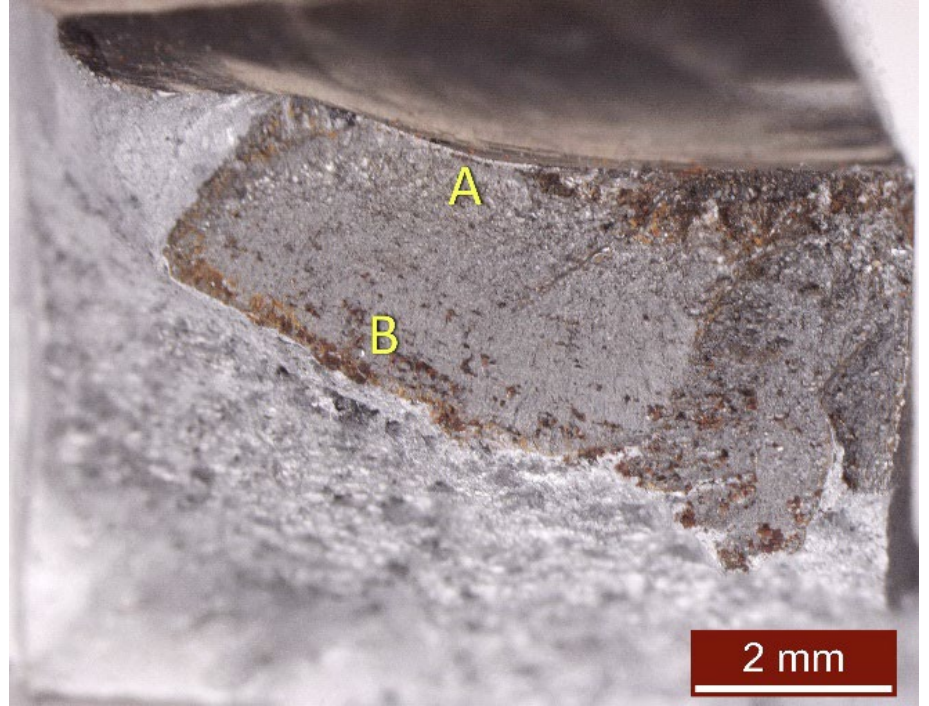
North Tie Girder (View looking South)



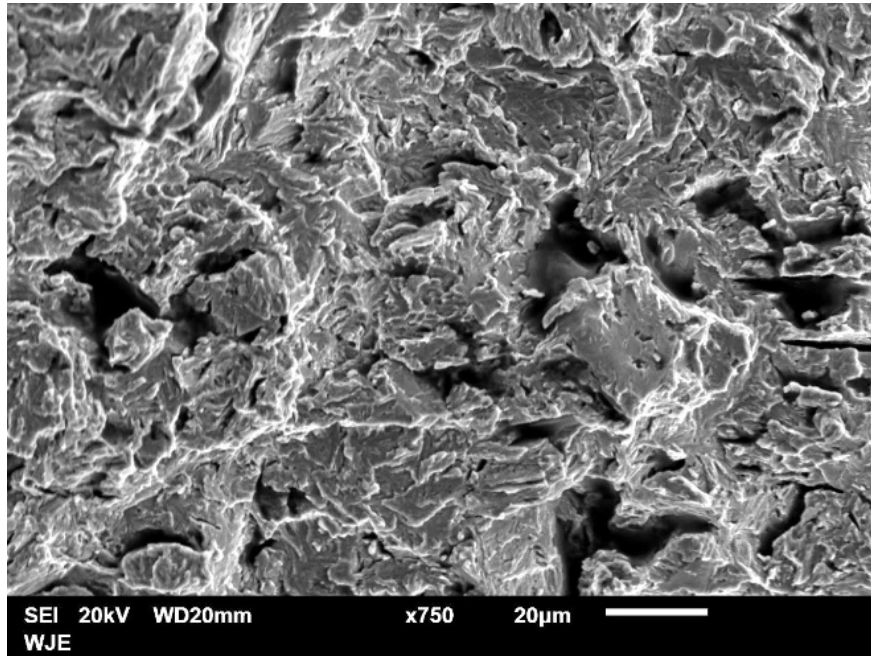
Core Extraction, Laboratory Evaluation, and Material Studies



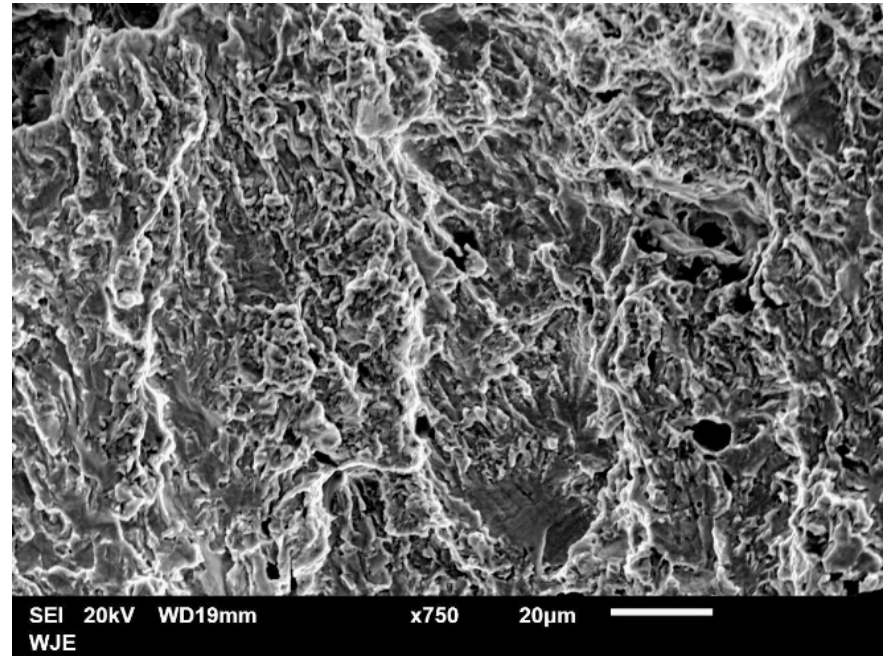




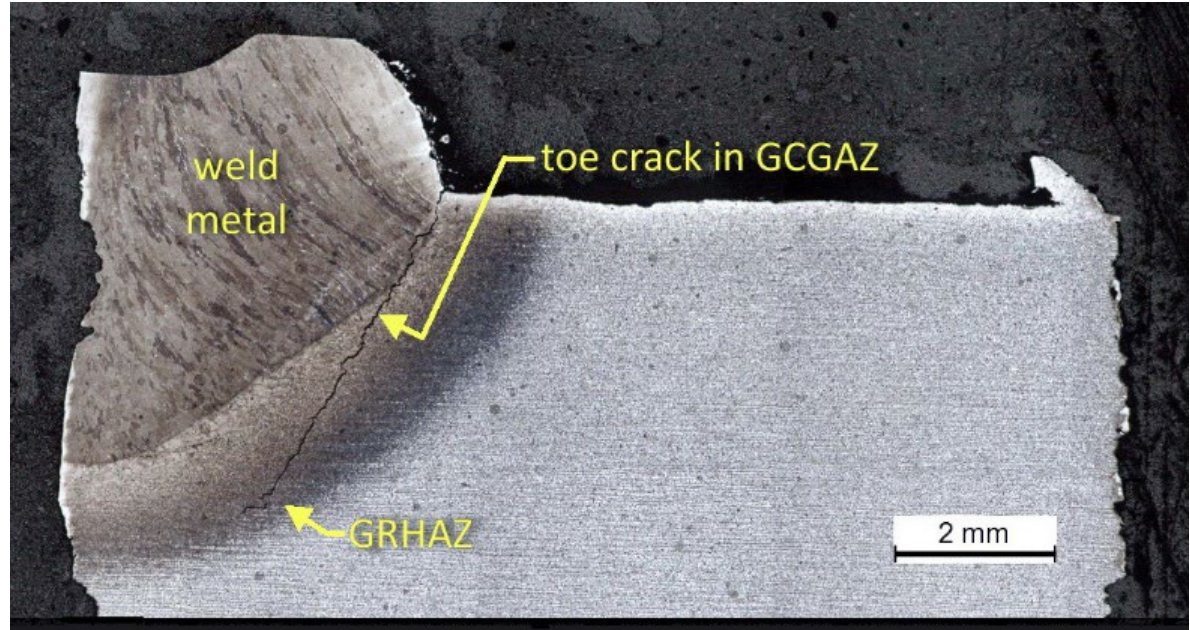
Surface A - GCHAZ



Surface B - GRHAZ



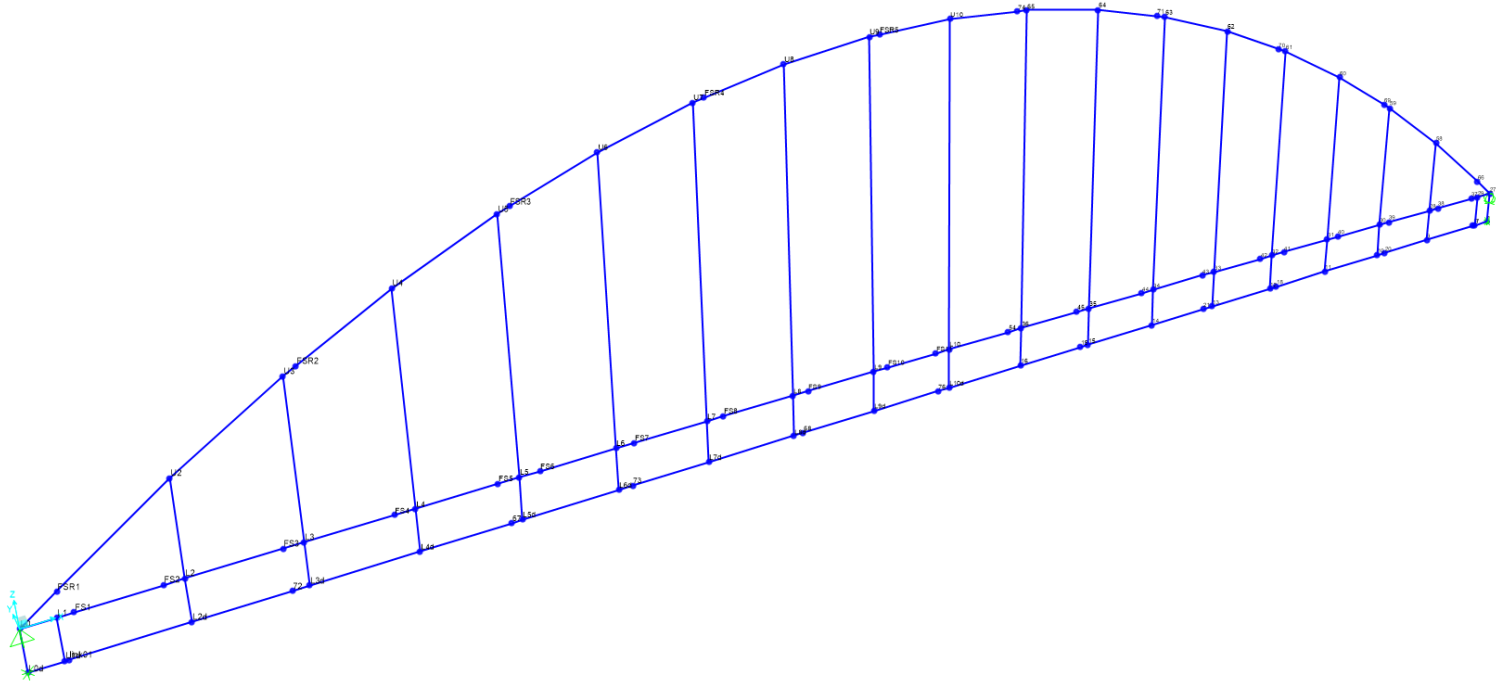
NE-80



Findings?

- No evidence of crack growth by fatigue.
- Bridge can be reopened to limited traffic.
- Cracks should be repaired to avoid brittle fracture

Structural Analyses

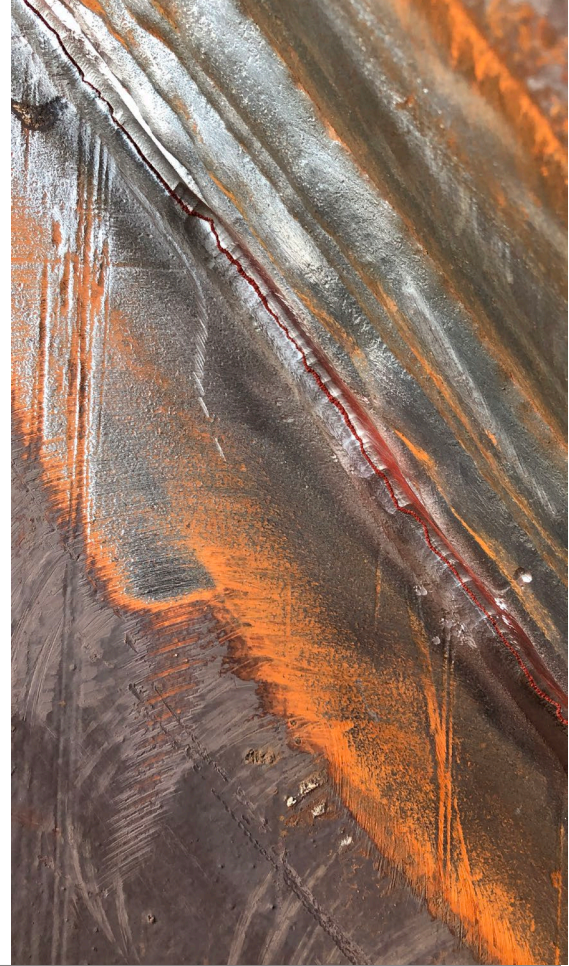


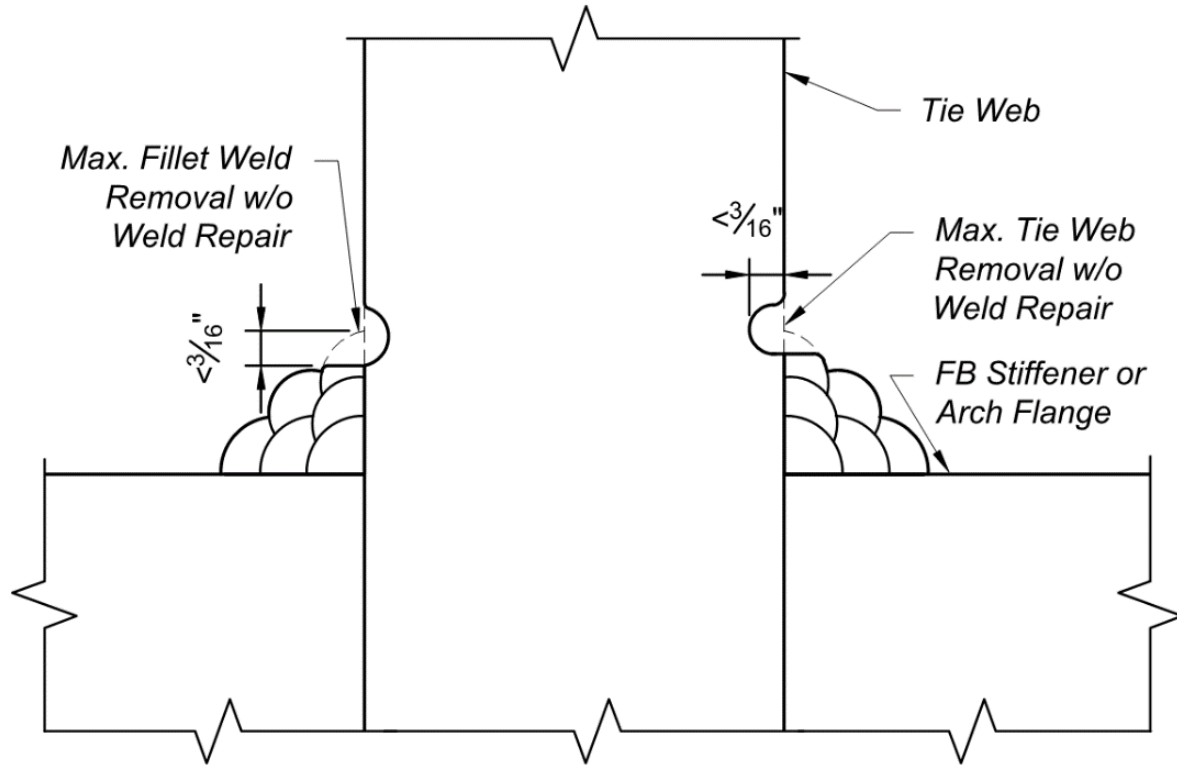
Weld Repair Process

1. Grind cracks to remove indications
2. Continue grinding as needed, verifying crack removed using MT
3. Preheat weld repair area to 300° F for minimum 1 hour
4. Install weld using SMAW with E7018 electrodes with H4R rating
5. MT root pass and maintain 300-450° F interpass temps.
6. MT intermittently and at completion of repair
7. Provide 3-hour bake-out at 300° F for a minimum of 3 hours
8. MT after cool down



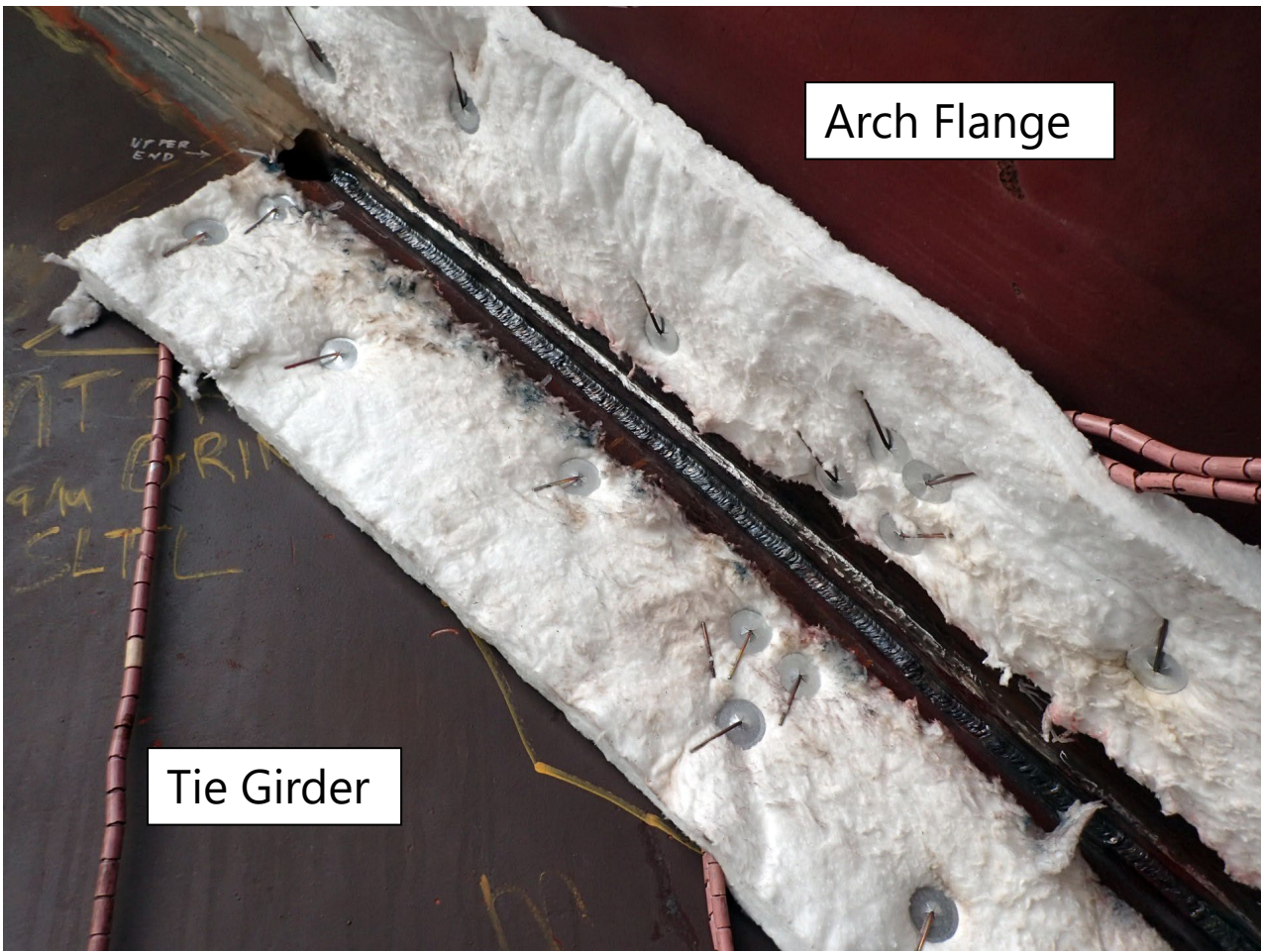












Arch Flange

Tie Girder

Conclusions

- Visual observations could not detect the cracks
- Hydrogen-induced cracking the likely cause
- No evidence of crack growth over nearly 40 years
- High restraint and plate thickness contributed
- No strict controls in place on base metal surface cleanliness or weld consumable moisture levels.
- Other structure constructed under FCP

Acknowledgments

- Missouri Department of Transportation
- Illinois Department of Transportation
- St. Louis Bridge Construction Company (General Contractor)
- Rednour Steel Erectors (Weld Repairs)
- TEAM Inc. (Heat Applicators)