



The Science You Build On.

FINANCIAL AND SCHEDULE BENEFITS OF PROJECT- SPECIFIC LOAD TESTING

109th Annual TEAM Conference

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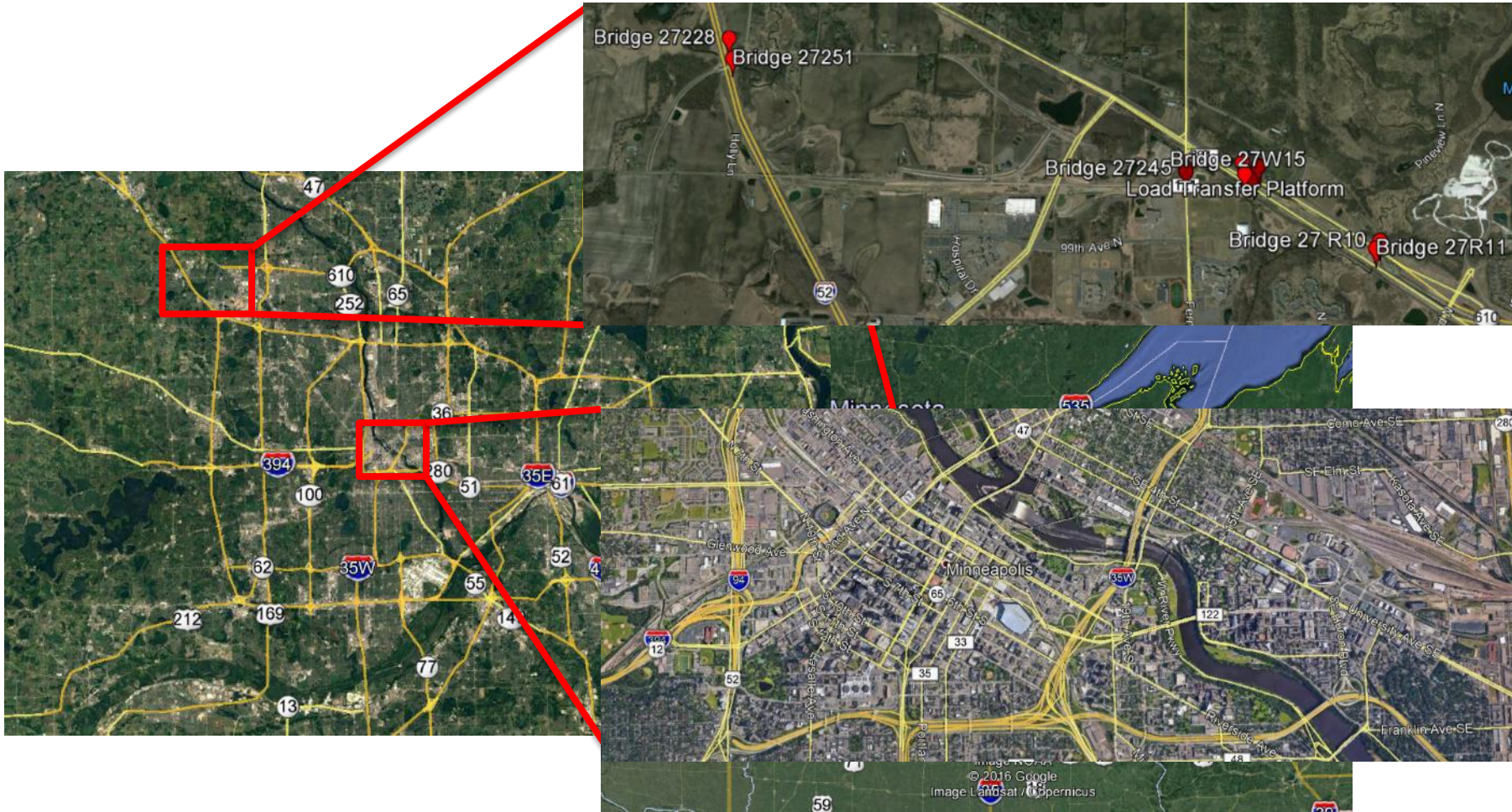
Introduction

- Foundation design process
- I-35W Bridge Replacement
- TH 610
- Conclusions

Foundation Design Process – Idealized!

- Structure need identified
- Preliminary structure design
- Subsurface exploration
 - Design-phase load test
- Final design
- Construction
 - Construction-phase load test

Project Locations



I-35W Bridge – Collapse



I-35W Bridge – Collapse



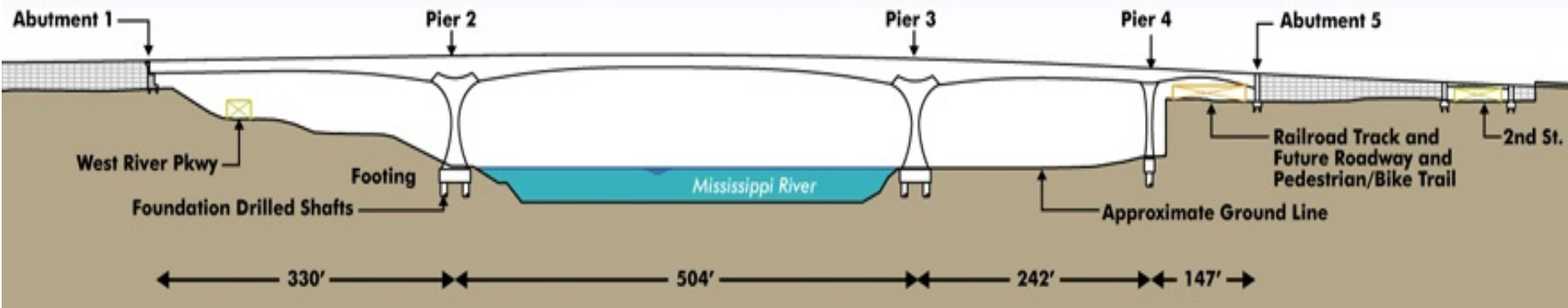
I-35W Bridge – Design

- Twin bridge replacement
- 125-year design life
- Overall length of 1,223 feet (373 m)
- Combined width of 176 feet (54 m)
- Foundation
 - Driven H-piles
 - Drilled shafts



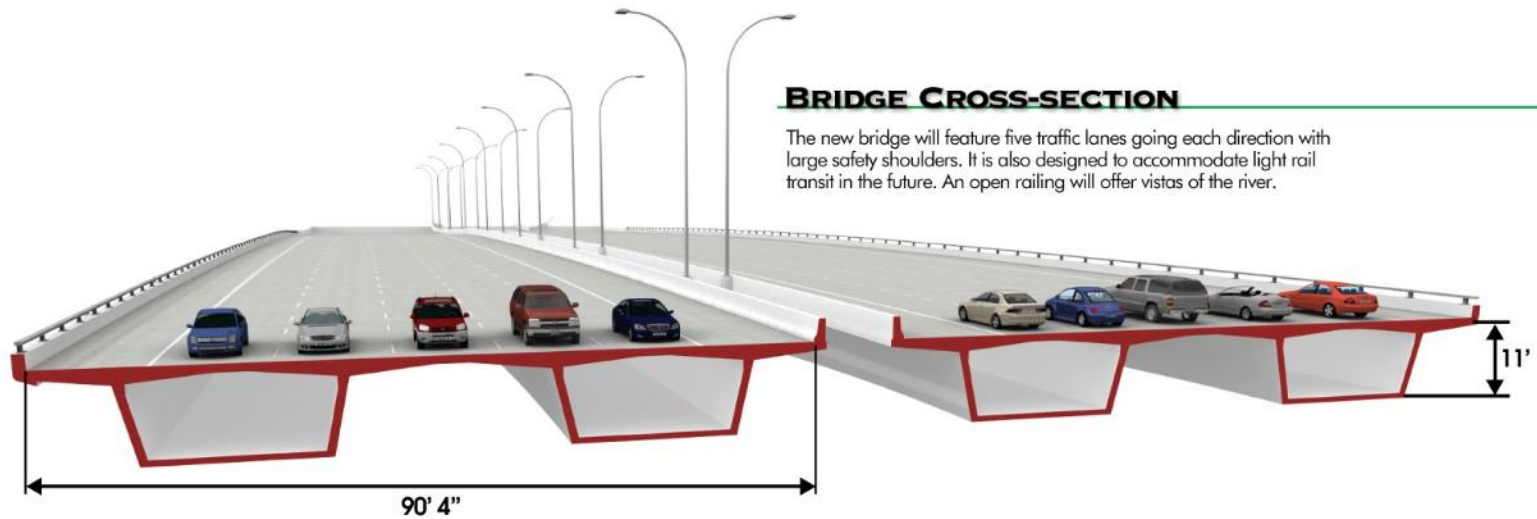
I-35W Bridge – Design

BRIDGE ELEVATION



BRIDGE CROSS-SECTION

The new bridge will feature five traffic lanes going each direction with large safety shoulders. It is also designed to accommodate light rail transit in the future. An open railing will offer vistas of the river.

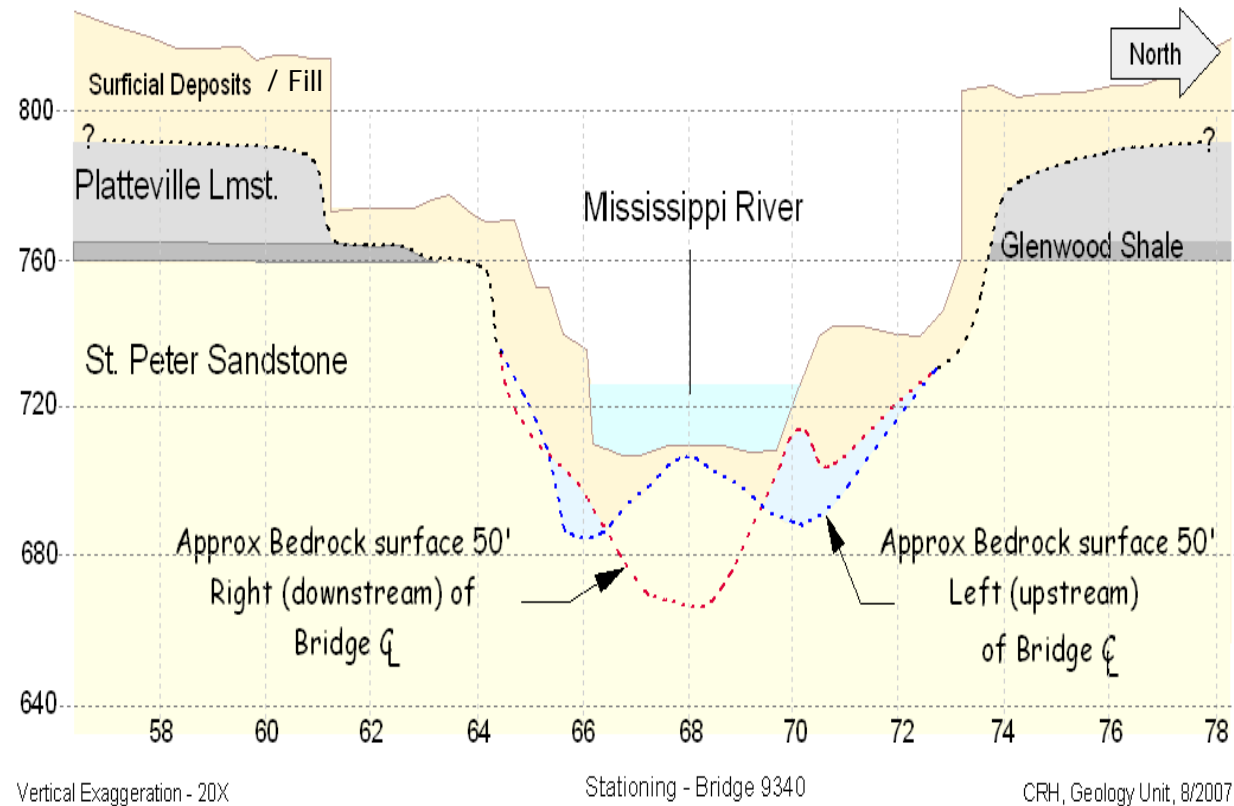


I-35W Bridge – Design



I-35W Bridge – Subsurface Conditions

- Primarily bedrock
- Artesian conditions
- Environmental challenges from previous development



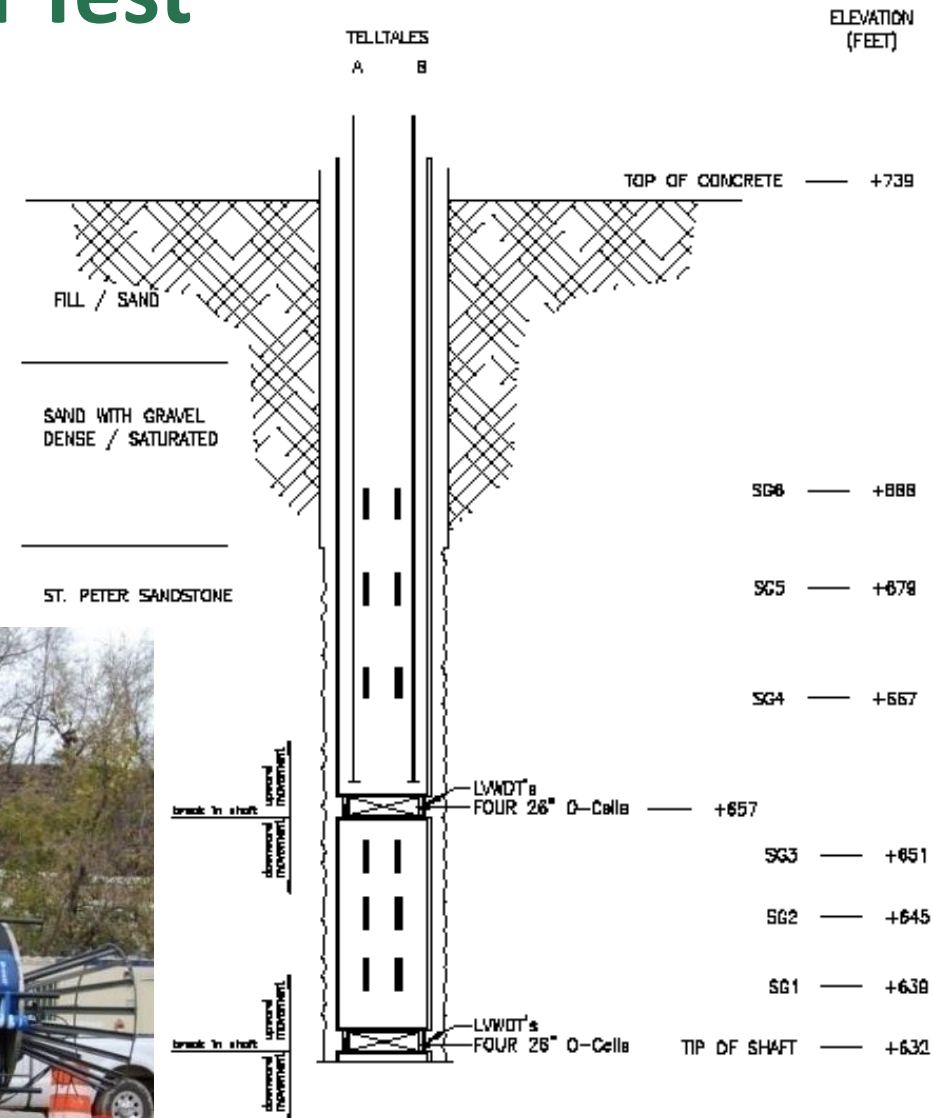
I-35W Bridge – Preliminary Shaft Design

- Rock Quality Designation (RQD) varied from 0% to 97%
- Unconfined compressive strength varied from 40 to 2,100 psi

Geotechnical Unit Resistance (ksf)			Rock Socket Diameter (inches)
Side Shear	End Bearing		
2 and 3	0.5 to 10	60 to 150	84
4			96

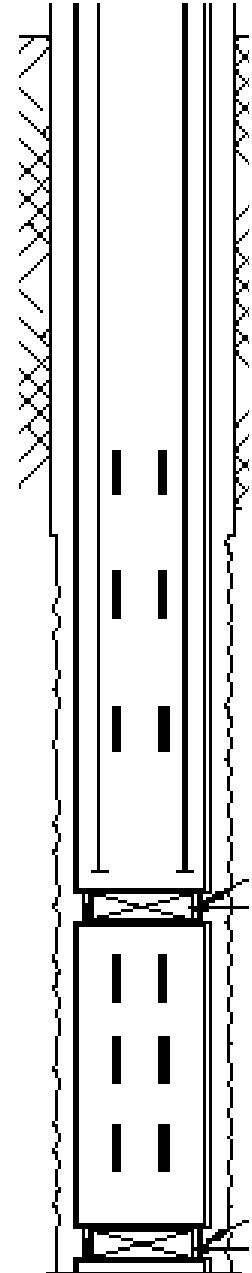
I-35W Bridge – Load Test

- Test and method shaft at Pier 3
- 78-inch-diameter, 39-foot-long, rock socket



I-35W Bridge – Load Test

- Two-level, three-stage, bi-directional load test
 1. Upper assembly closed, lower assembly pressurized
 2. Upper assembly pressurized, lower assembly open
 3. Upper assembly pressurized, lower assembly closed



I-35W Bridge – Load Test



I-35W Bridge – Unit Resistance Summary

Design Stage	Nominal Unit Resistance (ksf)	
	Side Shear	End Bearing
Initial	0.5 to 10 ksf	60 to 150 ksf
Test Shaft Design	2 to 8 ksf	150 ksf
Final (Test Shaft Actual)	2 to 40 ksf	90 ksf

- 400 to 2,500 percent increase in side shear resistance for more-competent sandstone
- End bearing resistance agrees with design estimates

I-35W Bridge – Final Shaft Design

	Pier 2	Pier 3	Pier 4
Initial Design Diameter (inches)	84	84	96
Final Design Diameter (inches)	78	90	90
Initial Design Socket Length (feet), overall / more-competent	136 / 68	124 / 41	215 / 171
Actual Socket Length (feet), overall / more-competent	54 / 22	50 / 23	80 / 16

I-35W Bridge – Final Shaft Design

Less-competent sandstone



More-competent sandstone



I-35W Bridge – Cost Comparison

- Drilling cost of \$45 per cubic foot in both soil and rock
- Cost of initial design: \$15,162,976
- Cost of final design:
 - Testing: \$583,000
 - Construction: \$7,726,612
 - Total: \$8,309,612
- Net savings resulting from testing: \$6,853,364

I-35W Bridge – Foundation Support Cost

$$A_{Con} \text{ Cap Support Cost} = \frac{\text{Cap Construction Cost}}{\sum \text{Factored Loads Under Construction Control}}$$

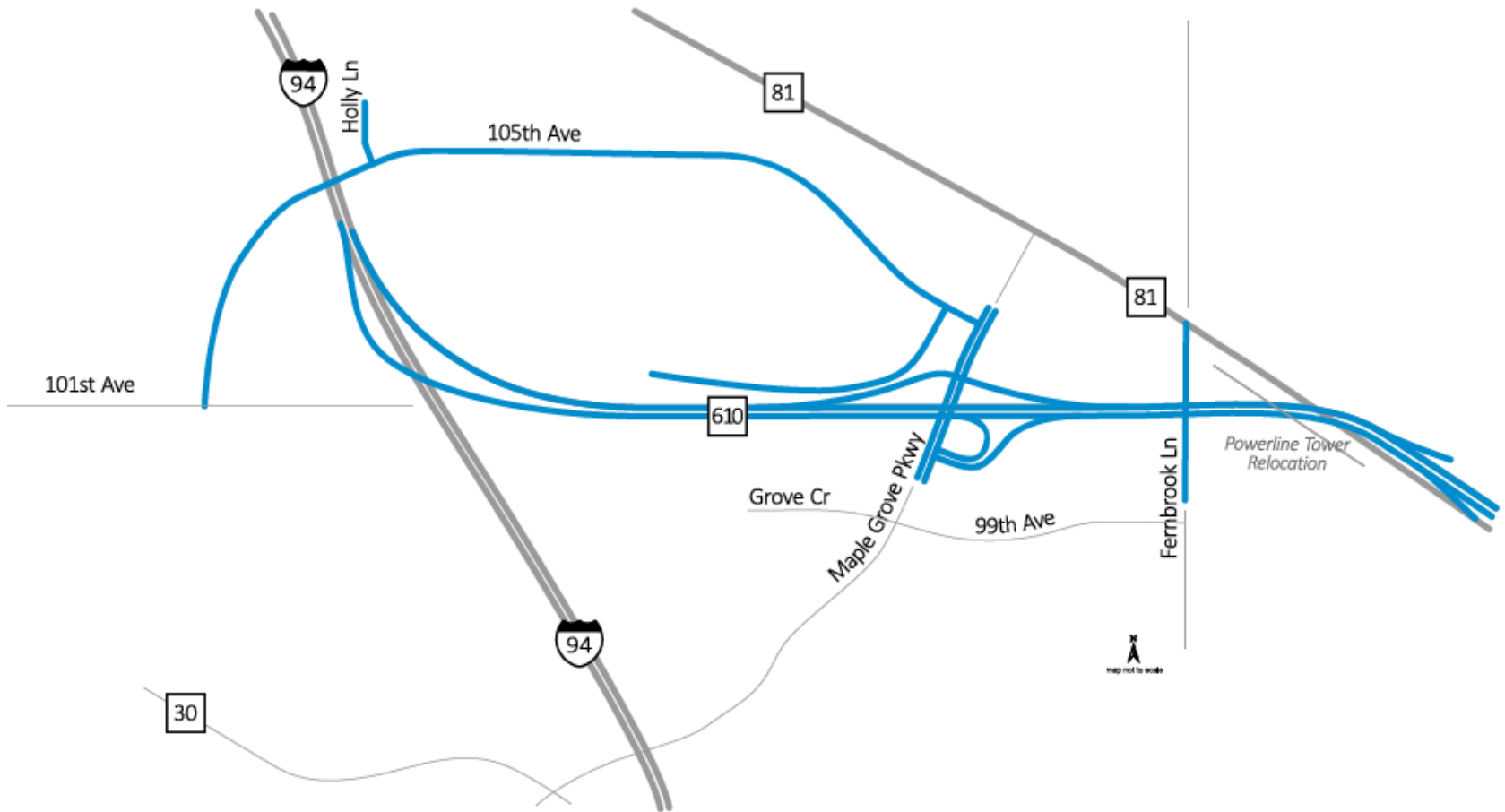
			Construction	
Total Support Cost			Total Foundation Cost	
			Structure Factored Loads	
Final	\$16.70/kip	\$20.09/kip	\$1.52/kip	\$21.61/kip

- Testing resulted in total support cost savings of \$17.81 per utilized kip of support

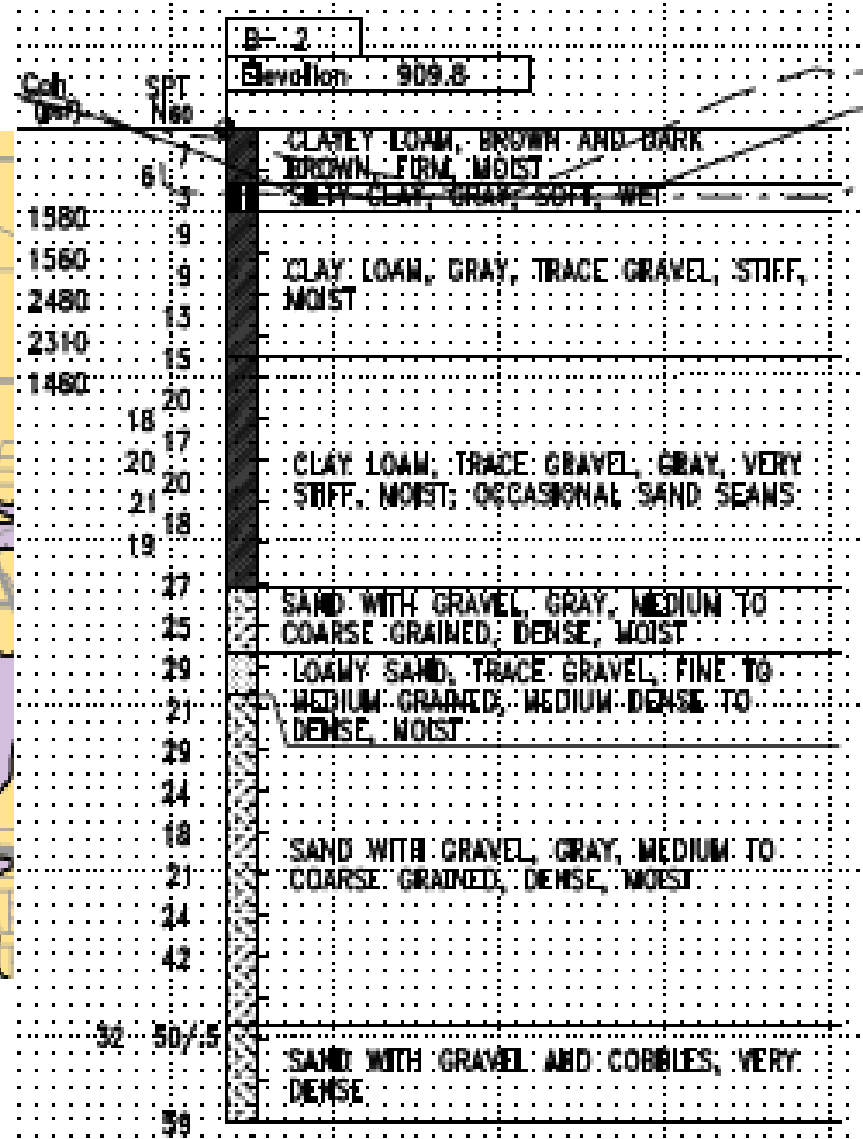
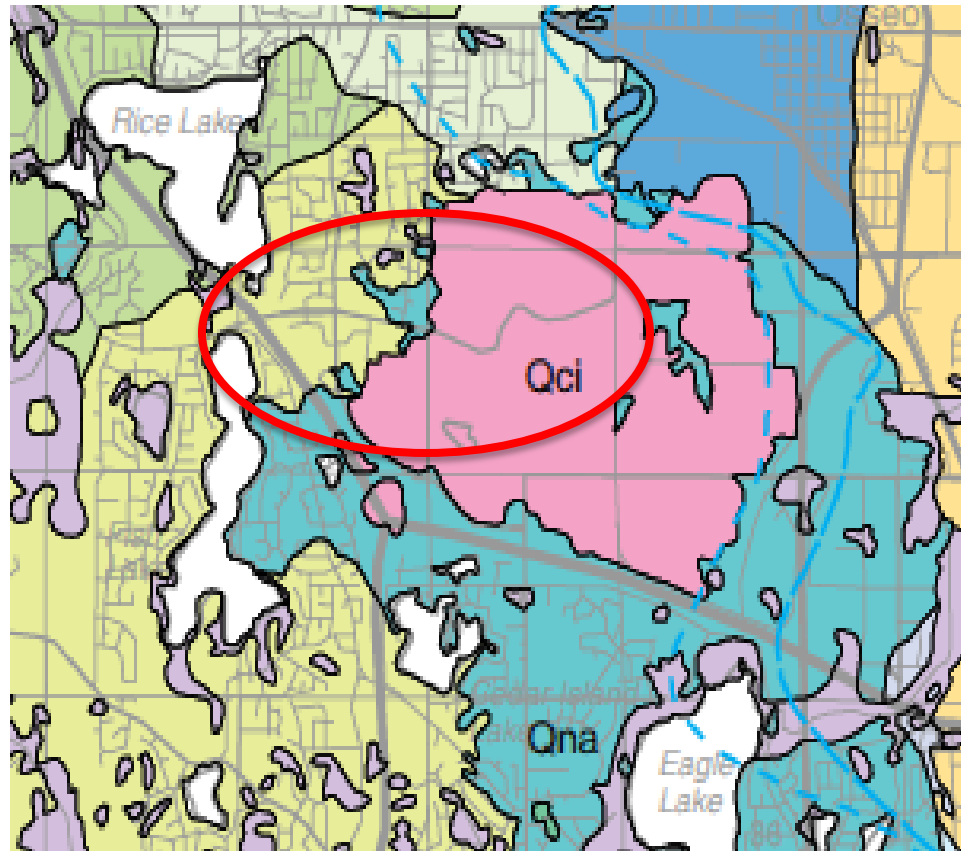
I-35W Bridge – Time Savings

- Initial design length (3,114 ft) – actual length (836 ft) = 2,278 feet of less drilling in more-competent rock
- Observed drilling rate of 1 to 4 feet/hour in more-competent rock means initial design would have required an additional 570 to 2,278 hours (23 to 95 days) of drilling

TH 610 Design



TH 610 – Subsurface Conditions



TH 610 – Foundation Design

Soil Type	Consistency	Blow Count, N_{60} (bpf)	Friction Angle, f (deg.)	Cohesion, c (psf)	β	N_t
Lean Clay (CL*)	Soft	2 - 4	-	250 - 500	0.15	3
	Firm	5 - 8	-	750 - 1,200	0.19	8
	Stiff	9 - 15	-	1,500 - 2,500	0.20 - 0.29	14 - 19
	Very Stiff	16 - 30	-	2,500 - 4,500	0.30 - 0.35	25 - 30
	Hard	31 - 60	-	4,500 - 9,000	0.36 - 0.40	30 - 33
	Very Hard	61+	-	10,000	0.41 - 0.50	37 - 40
Poorly Graded Sand (SP/SP-SM*)	Very Loose	0 - 4	28 - 29	-	0.15 - 0.20	15 - 20
	Loose	5 - 10	30 - 31	-	0.21 - 0.25	20 - 30
	Medium Dense	11 - 17	32 - 33	-	0.26 - 0.39	30 - 45
	Medium Dense	18 - 24	33 - 34	-	0.40 - 0.52	45 - 60
	Dense	25 - 30	35	-	0.53 - 0.59	60 - 75
	Dense	31 - 50	36 - 38	-	0.60 - 0.75	75 - 120
	Very Dense	51+	38 - 40	-	0.76 - 0.90	120 - 150

*Classification based on ASTM D2487 (2011).

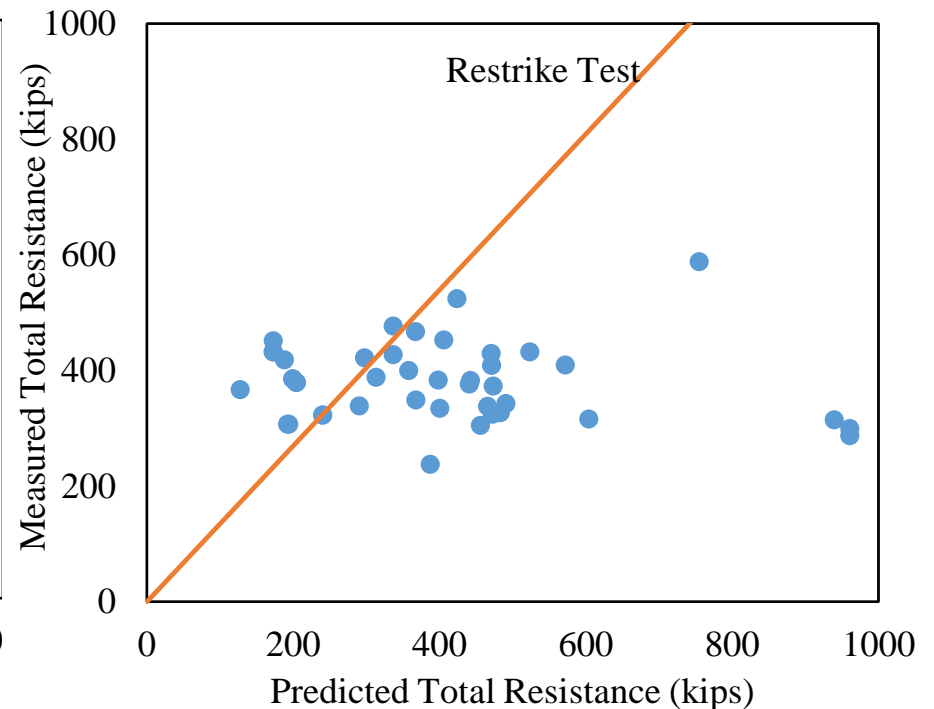
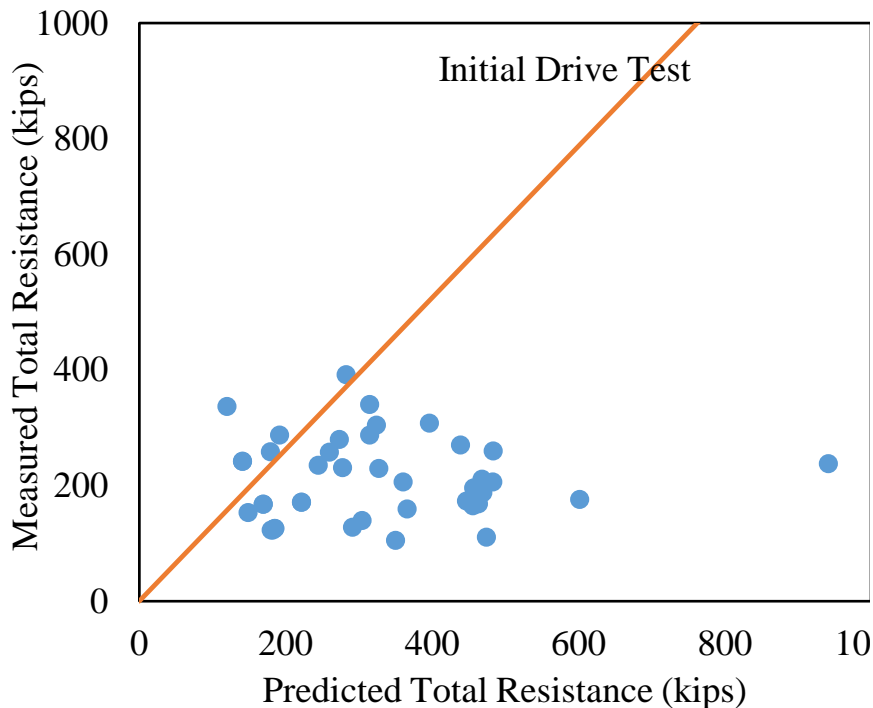
- Beta method, modified by experience

TH 610 – Pile Testing

- Closed-end pipe (CEP) piles
 - Diameter: 12 $\frac{3}{4}$ -inch
 - Wall thickness: $\frac{1}{4}$ -inch
- High-strain dynamic testing
 - Initial drive and restrike
 - Case method and wave matching using CAPWAP

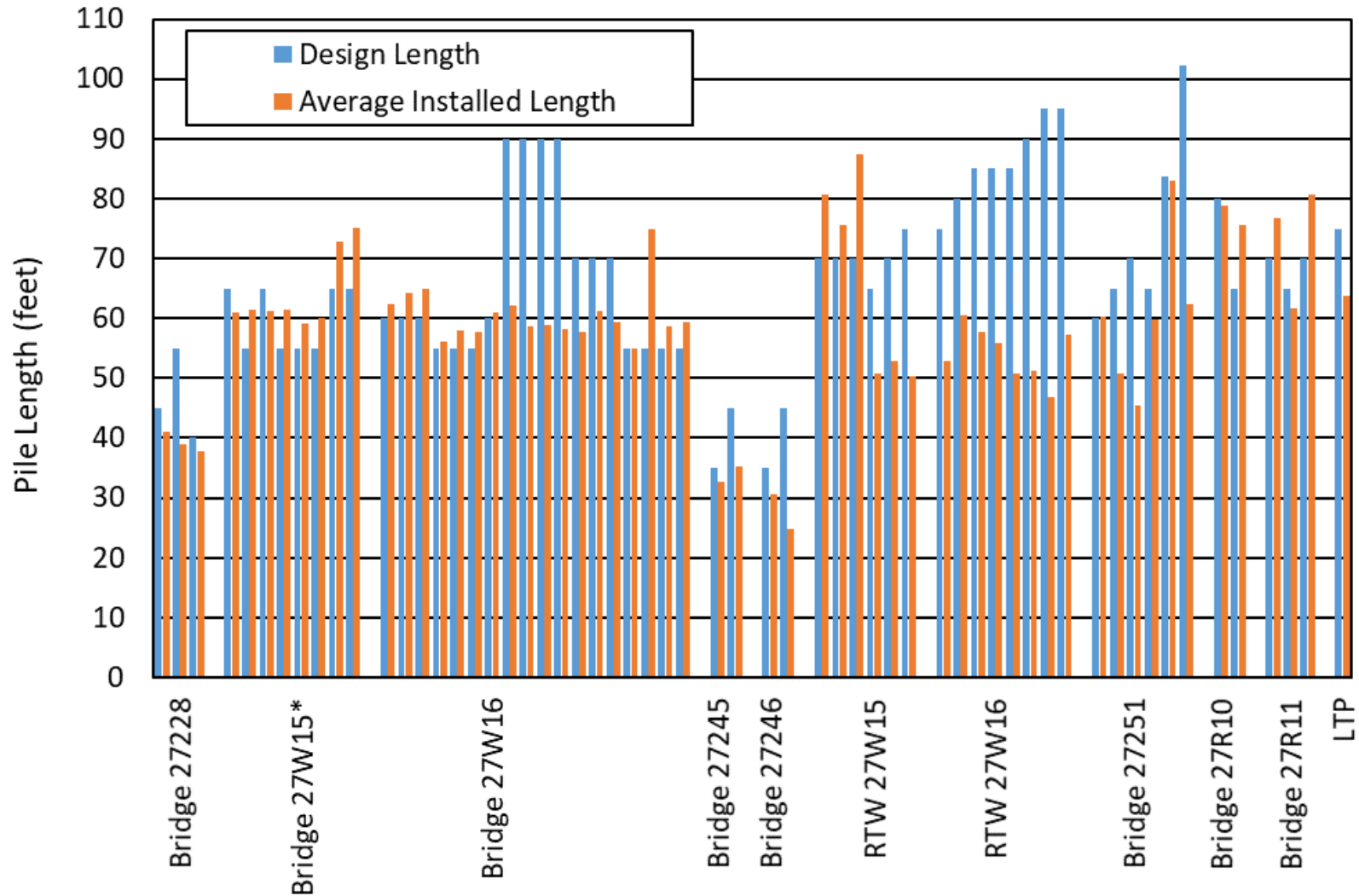
TH 610 – Pile Testing

- Test results versus prediction

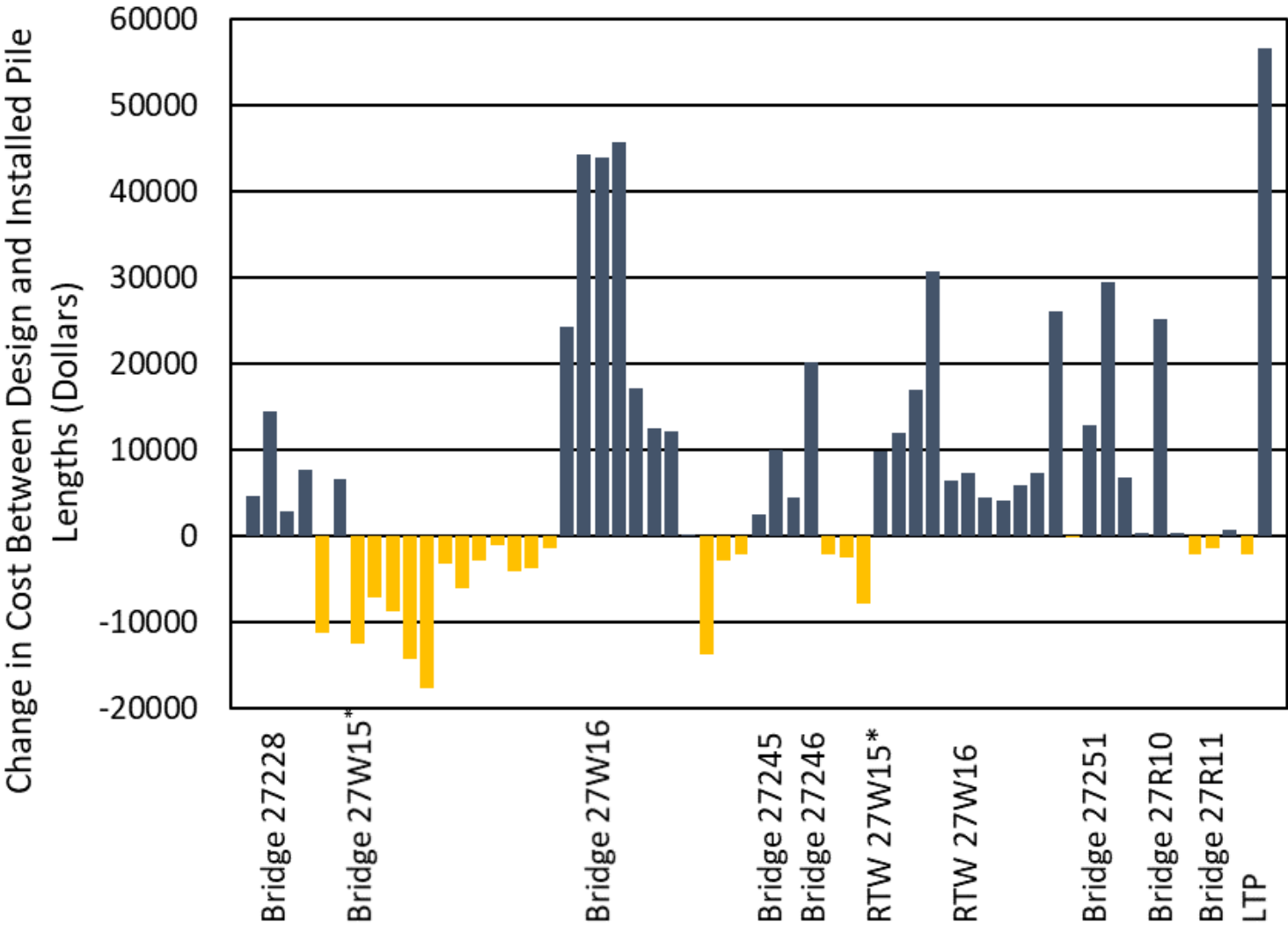


- Total bias of 0.81 for initial and 1.22 for restrike
- Side resistance bias of 0.83 and 1.56

TH 610 – Length Comparison



TH 610 – Costs



TH 610 – Costs

- Total design length less installed length is 13,548 ft
- Assume \$30/ft for savings of \$406,440
- High-strain dynamic testing fee \$51,584
- Estimated total savings of \$354,856
- For average pile length, saved approximately 28 days of driving

TH 610 – What does it mean?



- High-strain dynamic testing is more accurate than static analysis – maybe
 - Experience in static models
 - Over-estimate length for bidding
- Can't compare with lengths for formula or static load test

TH 610 – Conclusions

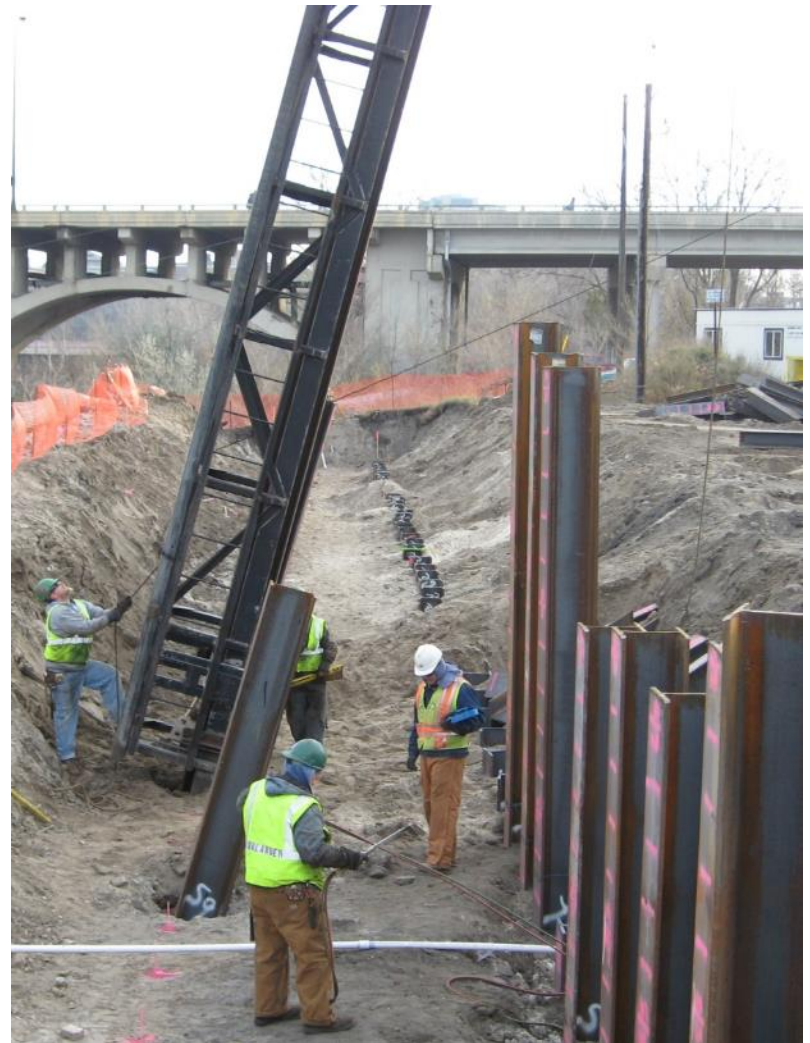
- Empirical methods are inaccurate, even with experience
- Restrike testing results in higher nominal resistance than initial-drive testing
- Foundation support cost analysis during design won't have all the information
- Foundation support cost analysis post-construction is also difficult with driven piles

I-35W Bridge – Summary

- Load testing cost \$1.52 per kip of utilized support
- Increased side shear resistance by 400 to 2,500 percent
- Testing saved \$17.81 per kip of utilized support and between 23 and 95 days of drilling

Both Projects – Conclusions

- Initial designs based on empirical values can be conservative
- Construction control with testing can be expensive, which can lead to easy dismissal
- Support cost provides a method of perspective



Both Projects – Conclusions



- Time-savings is important consideration that is not part of support cost
- Savings from test can be many times the total of testing cost

Acknowledgements



FLATIRON  MANSON



LUNDA

CONSTRUCTION CO.



Based on papers published and presented at IBC 2017 and GeoCongress 2017

Questions and Thank You!

